

MANDATARIA:



STUDIO D' INGEGNERIA ASSOCIATO
ISOLA-BOASSO & ASSOCIATI S.r.l.

MANDANTI:



ETATEC STUDIO PAOLETTI



C. & S. DI GIUSEPPE INGEGNERI
ASSOCIATI SRL Socio Unico

CIG: 896704821A

Vs. Rif. arch.:

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Ente destinatario:

-



ACQUA
NOVARA.VCO
S.p.A.

Via Triggiani, 9 - 28100 NOVARA (NO)
Tel. 0321/413111 - Fax. 0321/413196



PROGETTO ESECUTIVO

TITOLO COMMESSA

ADEGUAMENTO DEL DEPURATORE DI GRAVELLONA TOCE ALLE DIRETTIVE COMUNITARIE

Via Trattati di Roma in Comune di
Gravellona Toce (VB)

Rif. N° Commessa: W01M - 10030635
CUP: D49E17000030002
RUP: Dott. Ing Barbara Dell'Edera

Data: Maggio 2022

Rif. archivio: 002.19

Scala

ELABORATO: ST.01.004 G

Rev.	AGGIORNAMENTI	DATA

OGGETTO

FASCICOLO DEI CALCOLI – MANUFATTO PARTITORE AI
SEDIMENTATORI

Il Responsabile
Dott. Ing. Riccardo ISOLA

Visto

* Riservato all'Amministrazione

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1 Fascicolo dei calcoli Manufatto partitore ai Sedimentatori

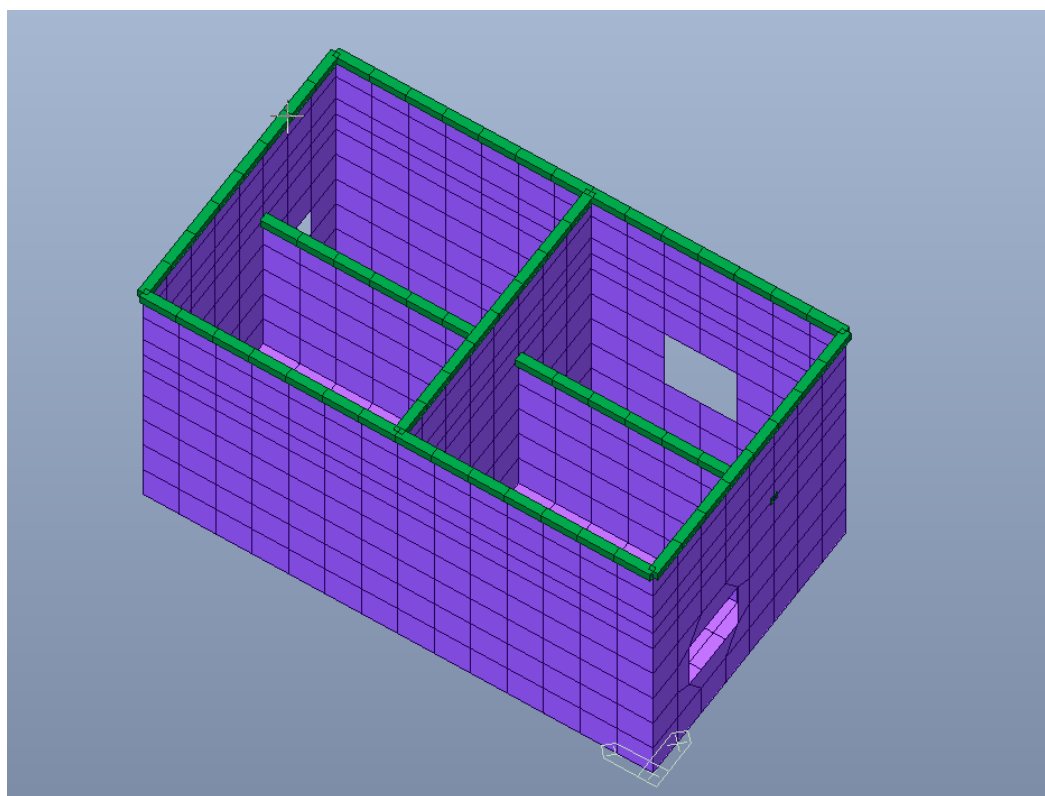
1.1 Dati della modellazione

Nell'immagine seguente viene riportato il modello di calcolo agli elementi finiti, realizzato con Midas Gen, con evidenziati i vari spessori degli elementi strutturali.

Gli elementi finiti adottati sono di tipo plate per le pareti e beam per le travi.

In particolare, gli spessori adottati per gli elementi plate sono i seguenti:

- Platea di fondazione sp.30 cm;
- Pareti esterne verso sp.30 cm;
- Parete interna alta sp.30 cm;
- Pareti interne per stramazzi sp.20 cm.



Modello di Calcolo agli elementi finiti

L'interazione terreno struttura a livello del piano di fondazione è stata modellata con delle molle alla Winkler con costante di sottofondo media pari a circa 16000 kN/m^3 , ricavata dalle caratteristiche geotecniche del terreno con la formula di Vesic.

Essendo l'opera praticamente tutta interrata, l'analisi sismica è stata effettuata mediante analisi statica equivalente, nella quale le azioni sismiche sono state inserite come sovra spinte sismiche sia del terreno che del liquame, calcolate rispettivamente con le formule di Wood e di Housner.

Per la determinazione delle suddette spinte si faccia riferimento alla relazione di calcolo strutturale.

1.2 Carichi e combinazioni di carico

Per la determinazione delle azioni sul manufatto si faccia riferimento alla relazione strutturale.

Nel presente paragrafo vengono riportati i carichi assegnati ai vari elementi strutturali, le condizioni di carico elementari considerate e le combinazioni di carico.

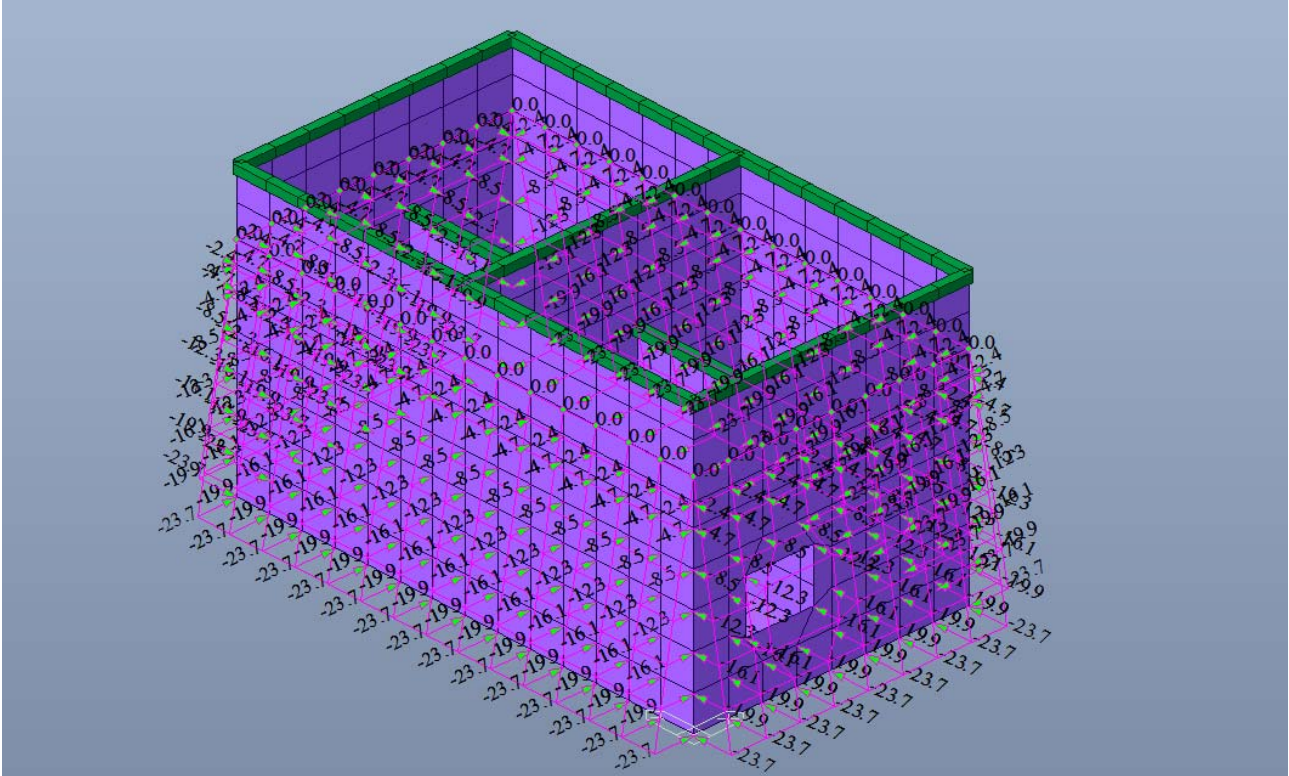
Le condizioni di carico adottate nella modellazione della struttura sono le seguenti:

No	Name	Type	Description
1	DL	Dead Load (D)	Peso Proprio
2	PP	Dead Load (D)	Permanente Paratoie
3	PG	Dead Load (D)	Permanente Grigliati
4	PM	Dead Load (D)	Permanente Magrone
5	VM	Live Load (L)	Variabile Manutenzione
6	SST	Dead Load (D)	Spinta Statica Terreno
7	SSTF	Dead Load (D)	Spinta Statica Terreno con Falda
8	SSL1	Dead Load (D)	Spinta Statica Liquame 1
9	SSL2	Dead Load (D)	Spinta Statica Liquame 2
10	SGAL	Dead Load (D)	Spinta Galeggiamento
11	SSS	Live Load (L)	Spinta Sovraccarico
12	ET X	Earthquake (E)	Spinta Sismica Terreno X
13	ET Y	Earthquake (E)	Spinta Sismica Terreno Y
14	EL X	Earthquake (E)	Spinta Sismica Liquame X
15	EL Y	Earthquake (E)	Spinta Sismica Liquame Y

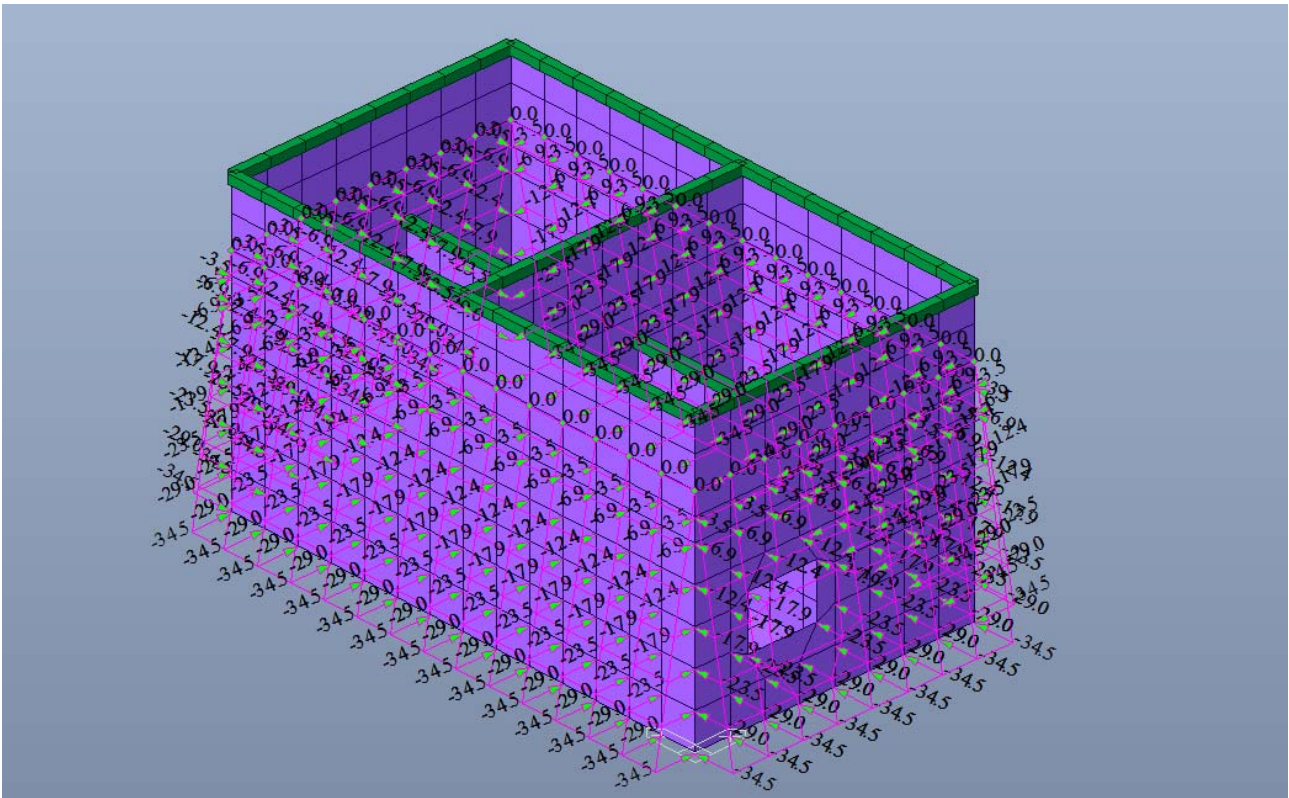
Le combinazioni delle condizioni di carico elementari, realizzate sulla base delle indicazioni del paragrafo 2.5.3 delle NTC 2018, sono le seguenti:

No	Name	DL(ST)	PP(ST)	PG(ST)	PM(ST)	VM(ST)	SST(ST)	SSTF(ST)	SSL1(ST)	SSL2(ST)	SGAL(ST)	SSS(ST)	ET X(ST)	ET Y(ST)	EL X(ST)	EL Y(ST)
1	SLU 1	1.3000	1.3000	1.3000	1.3000	1.5000	1.3000									
2	SLU 2	1.3000	1.3000	1.3000	1.3000	1.5000		1.3000			1.3000					
3	SLU 3	1.3000	1.3000	1.3000	1.3000	1.5000	1.3000		1.3000							
4	SLU 4	1.3000	1.3000	1.3000	1.3000	1.3000						1.5000				
5	SLU 5	1.3000	1.3000	1.3000	1.3000	1.5000	1.3000		1.3000			1.0500				
6	SLU 6	1.3000	1.3000	1.3000	1.3000	1.5000		1.3000	1.3000							
7	SLU 7	1.3000	1.3000	1.3000	1.3000	1.5000		1.3000	1.3000			1.0500				
8	SLU 8	1.3000	1.3000	1.3000	1.3000	1.0500		1.3000	1.3000			1.5000				
9	SLV 1	1.0000	1.0000	1.0000	1.0000		1.0000						1.0000	0.3000		
10	SLV 2	1.0000	1.0000	1.0000	1.0000		1.0000						1.0000	-0.3000		
11	SLV 3	1.0000	1.0000	1.0000	1.0000		1.0000						-1.0000	0.3000		
12	SLV 4	1.0000	1.0000	1.0000	1.0000		1.0000						-1.0000	-0.3000		
13	SLV 5	1.0000	1.0000	1.0000	1.0000		1.0000						0.3000	1.0000		
14	SLV 6	1.0000	1.0000	1.0000	1.0000		1.0000						0.3000	-1.0000		
15	SLV 7	1.0000	1.0000	1.0000	1.0000		1.0000						-0.3000	1.0000		
16	SLV 8	1.0000	1.0000	1.0000	1.0000		1.0000						-0.3000	-1.0000		
17	SLV 9	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			1.0000	0.3000	1.0000	0.3000
18	SLV 10	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			1.0000	-0.3000	1.0000	-0.3000
19	SLV 11	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			-1.0000	0.3000	-1.0000	0.3000
20	SLV 12	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			-1.0000	-0.3000	-1.0000	-0.3000
21	SLV 13	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			0.3000	1.0000	0.3000	1.0000
22	SLV 14	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			0.3000	-1.0000	0.3000	-1.0000
23	SLV 15	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			-0.3000	1.0000	-0.3000	1.0000
24	SLV 16	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000			-0.3000	-1.0000	-0.3000	-1.0000
25	SLE R1	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000									
26	SLE R2	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000					
27	SLE R3	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000							
28	SLE R4	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000					1.0000				
29	SLE R5	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000			0.7000				
30	SLE R6	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000							
31	SLE R7	1.0000	1.0000	1.0000	1.0000	1.0000		1.0000	1.0000			0.7000				
32	SLE R8	1.0000	1.0000	1.0000	1.0000	0.5000		1.0000	1.0000			1.0000				
33	SLE F1	1.0000	1.0000	1.0000	1.0000	0.5000	1.0000									
34	SLE F2	1.0000	1.0000	1.0000	1.0000	0.5000	1.0000					0.3000				
35	SLE F3	1.0000	1.0000	1.0000	1.0000	0.5000	1.0000			1.0000						
36	SLE F4	1.0000	1.0000	1.0000	1.0000	0.5000	1.0000			1.0000		0.3000				
37	SLE Qp	1.0000	1.0000	1.0000	1.0000		1.0000			1.0000						

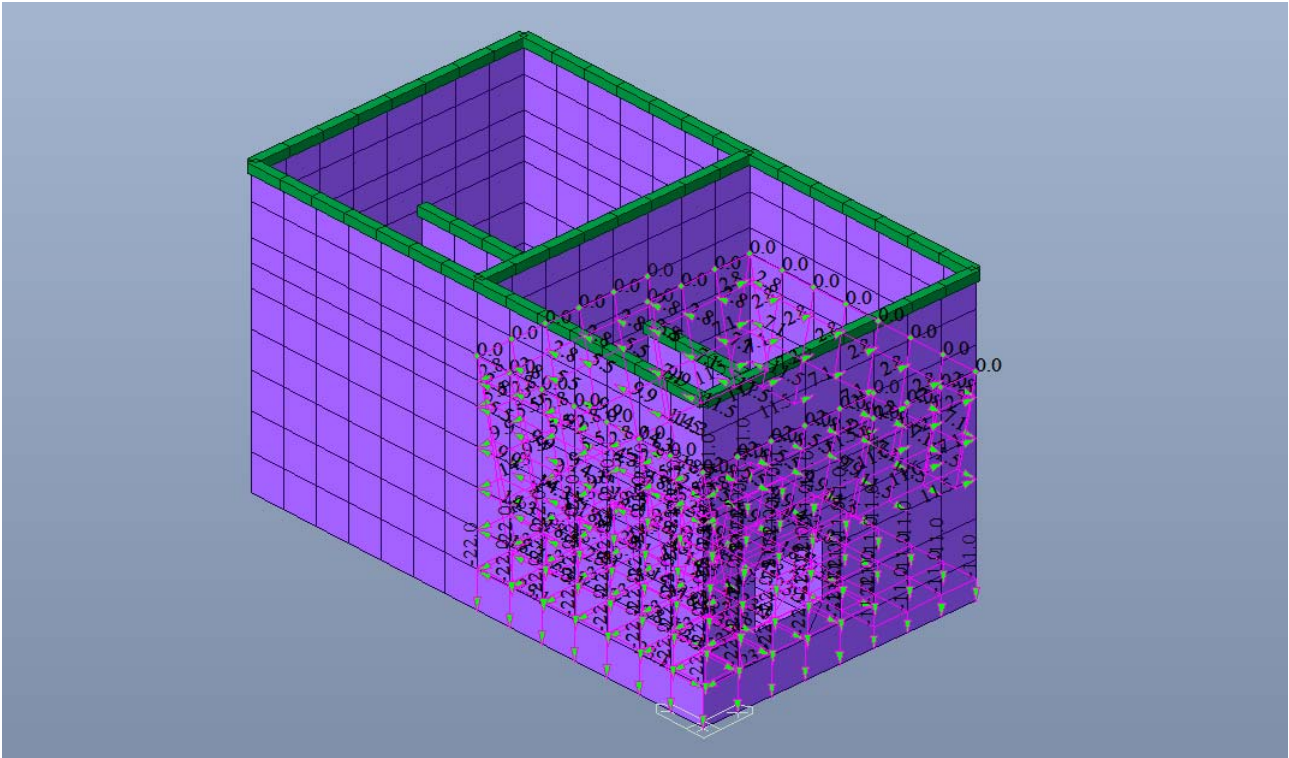
Nelle immagini seguenti si riportano le assegnazioni dei carichi:



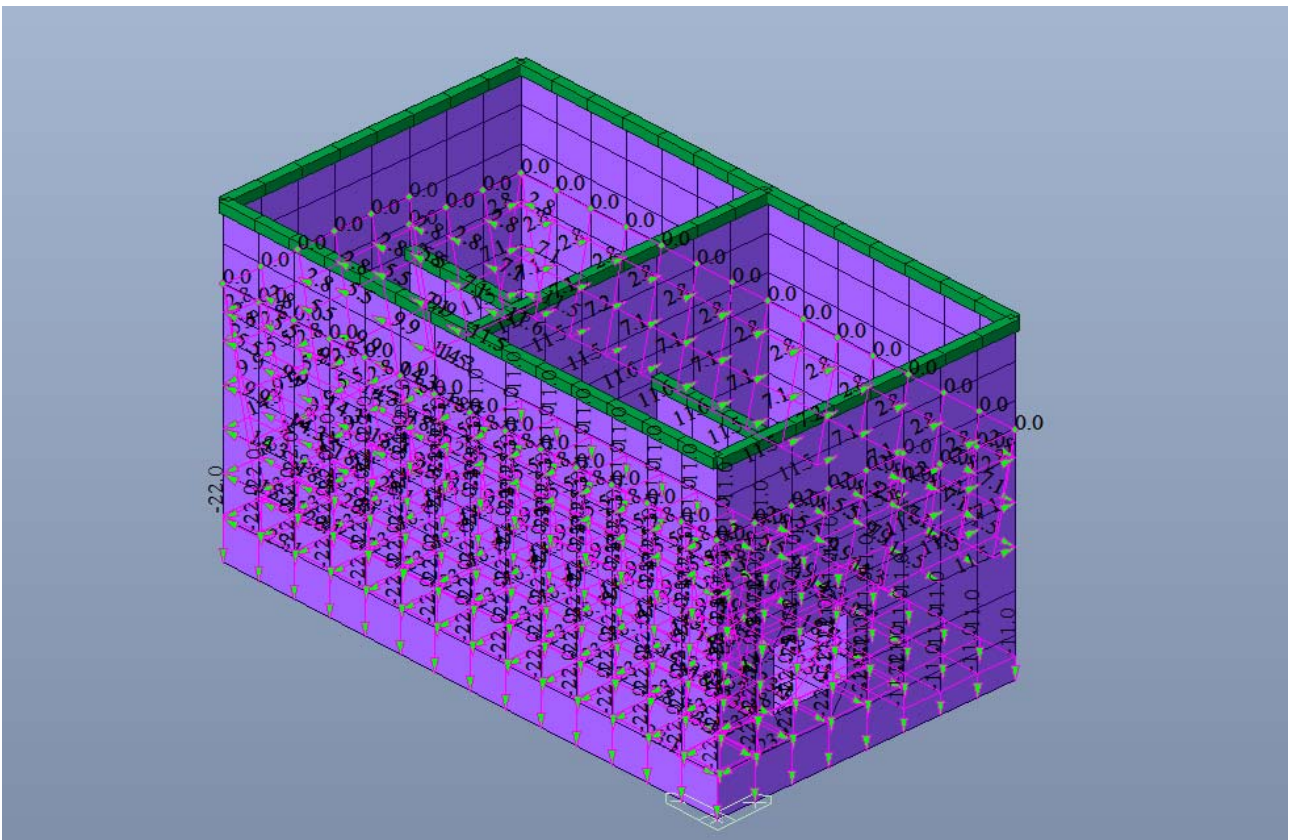
Assegnazione spinta statica del terreno (SST) - [kN/mq]



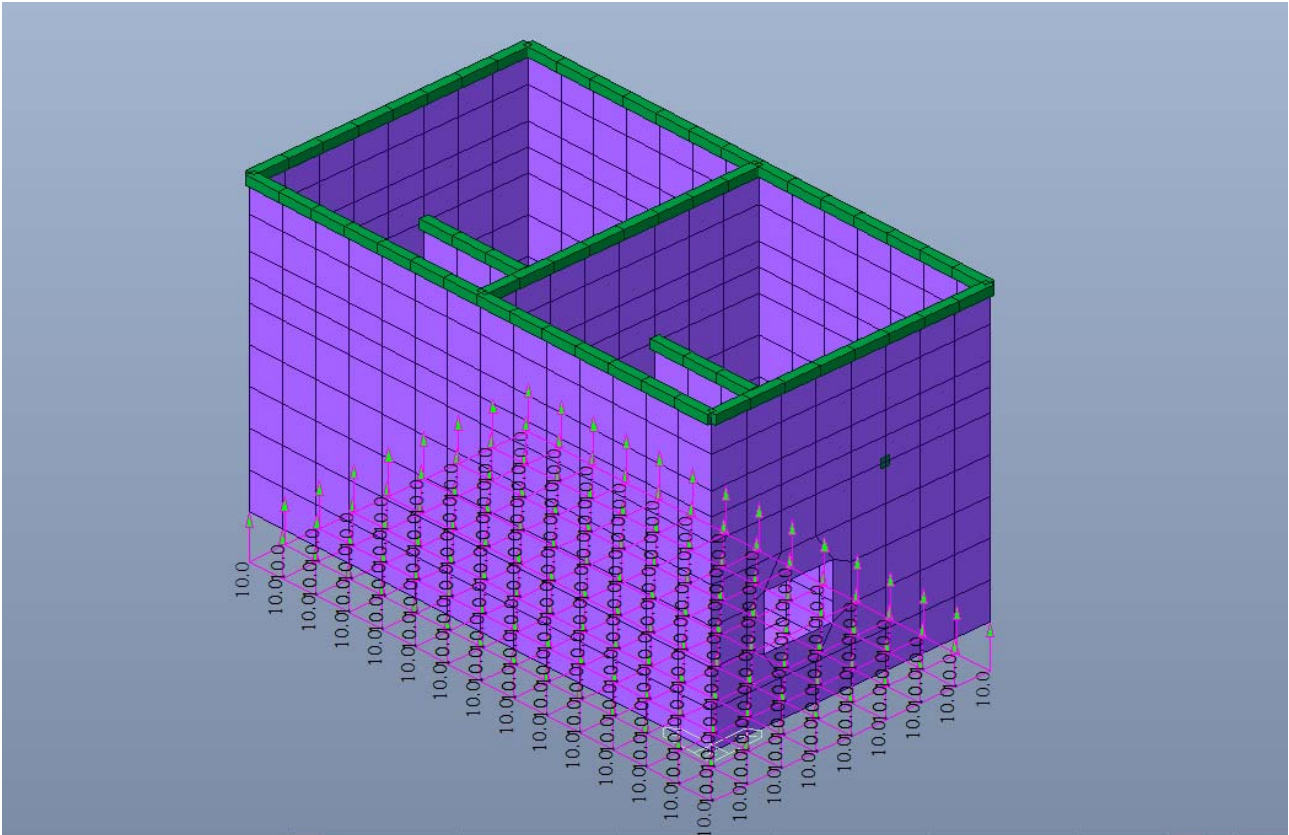
Assegnazione spinta statica del terreno con falda (SSTF) - [kN/mq]



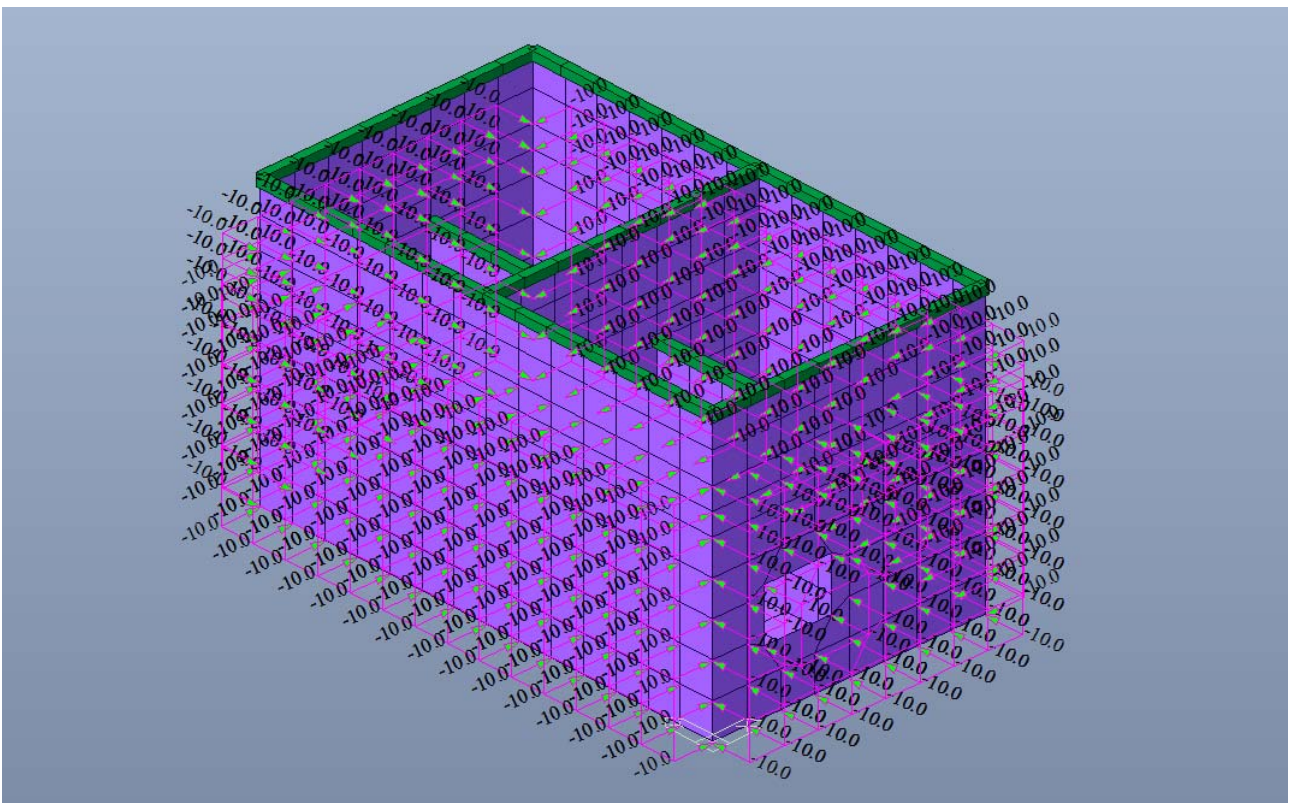
Assegnazione spinta statica liquame una vasca (SSL1) - [kN/mq]



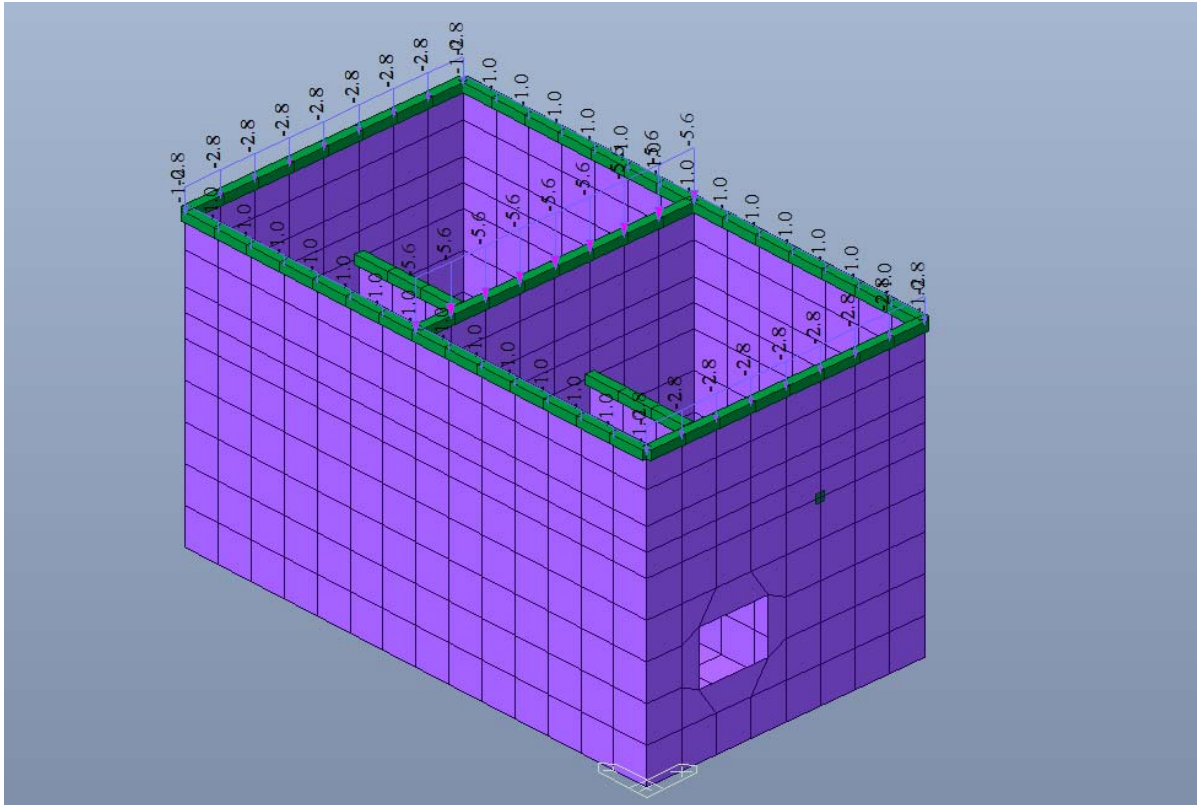
Assegnazione spinta statica liquame entrambe le vasche (SSL2) - [kN/mq]



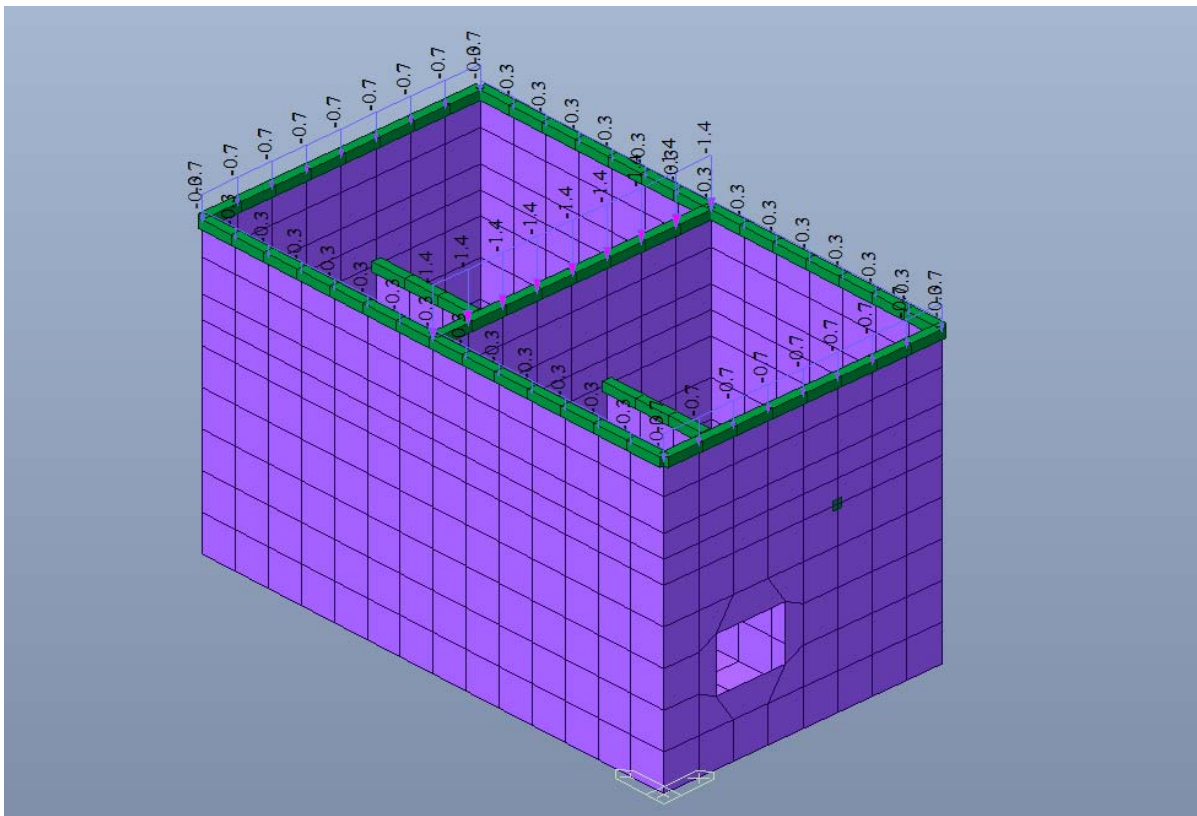
Assegnazione spinta galleggiamento (S GAL) - [kN/mq]



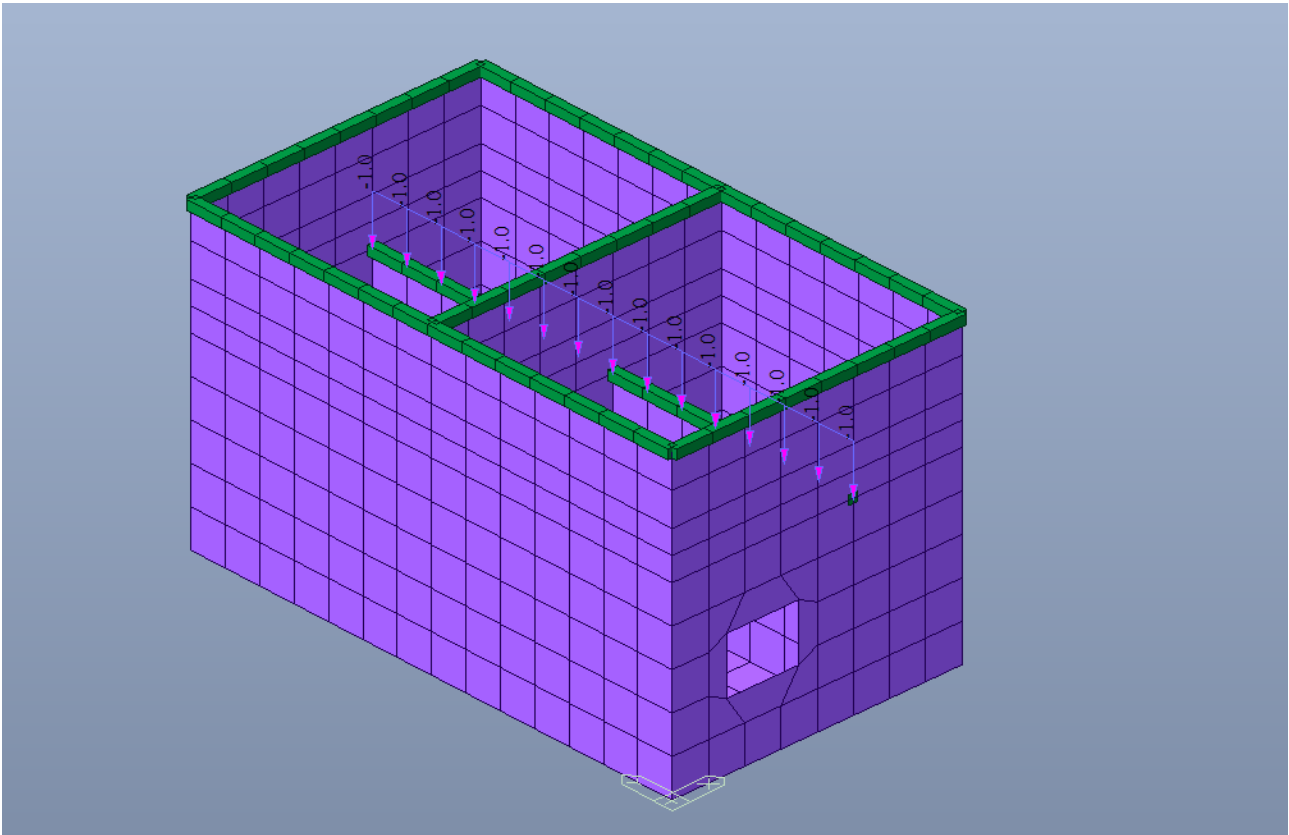
Assegnazione spinta statica sovraccarico (SSS) - [kN/mq]



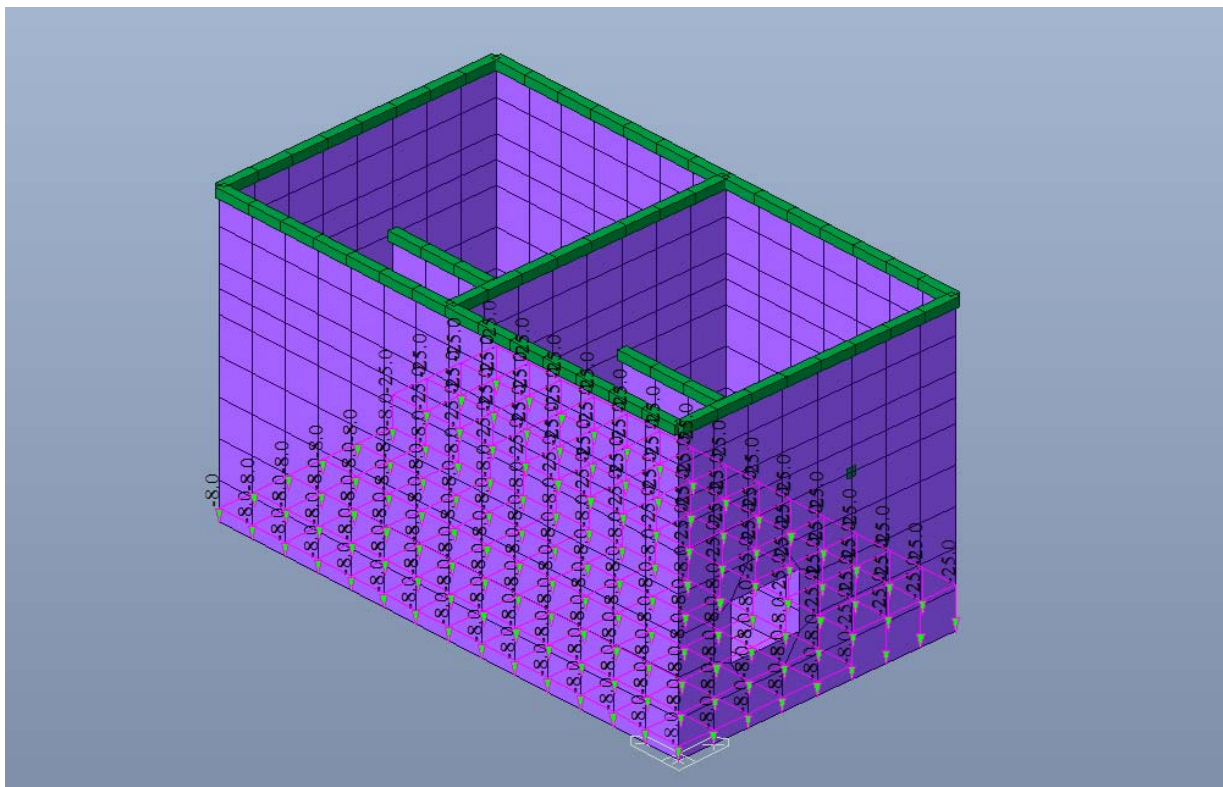
Assegnazione variabile manutenzione (VM) - [kN/m]



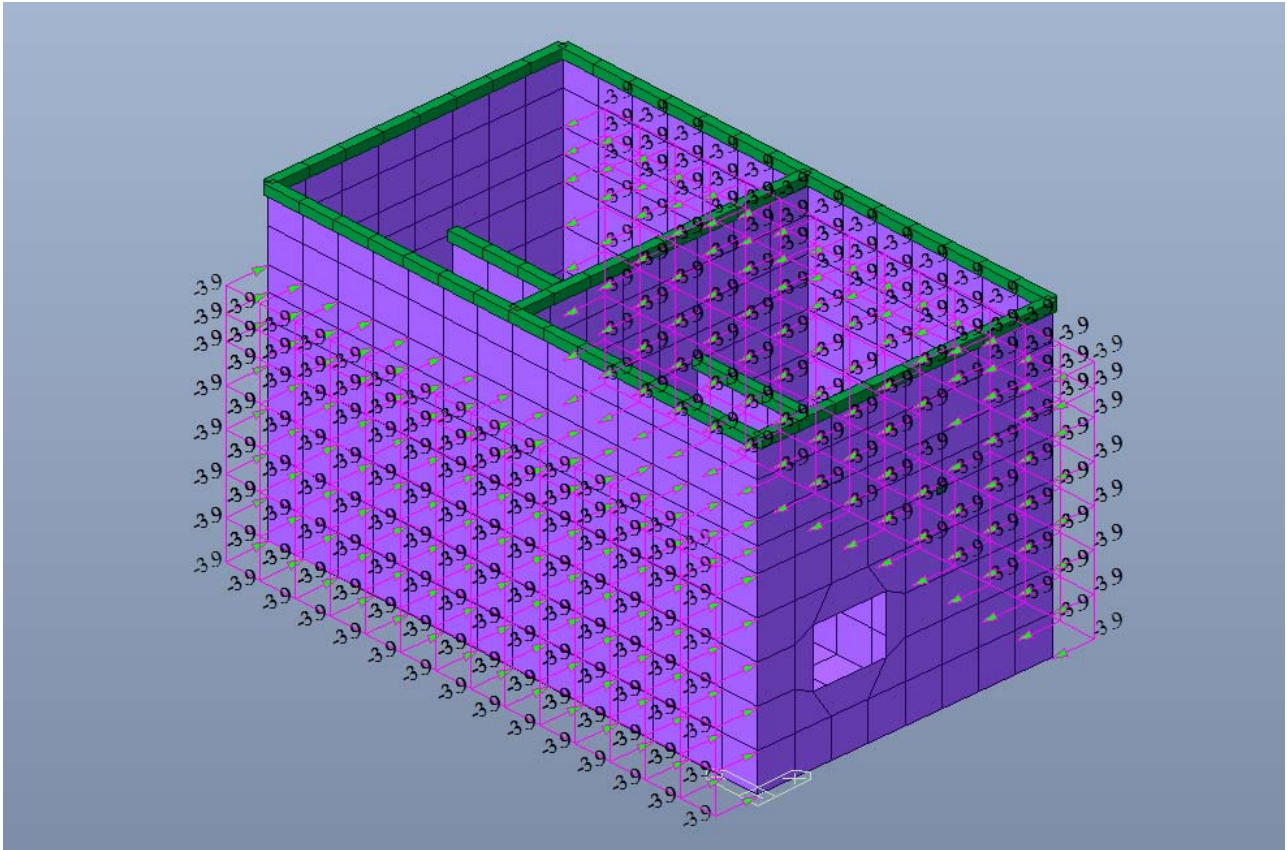
Assegnazione permanente grigliati (PG) - [kN/m]



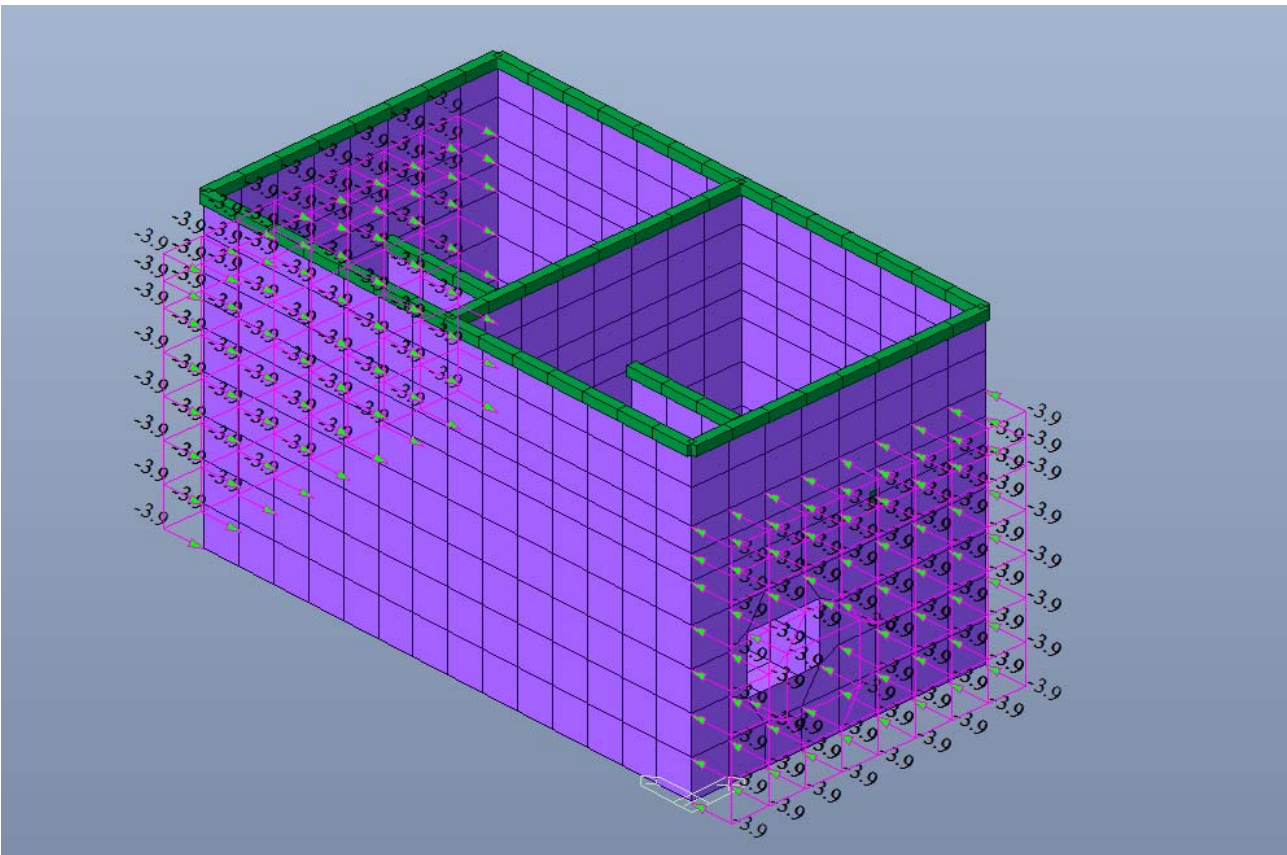
Assegnazione permanente paratoie (PP) - [kN/m]



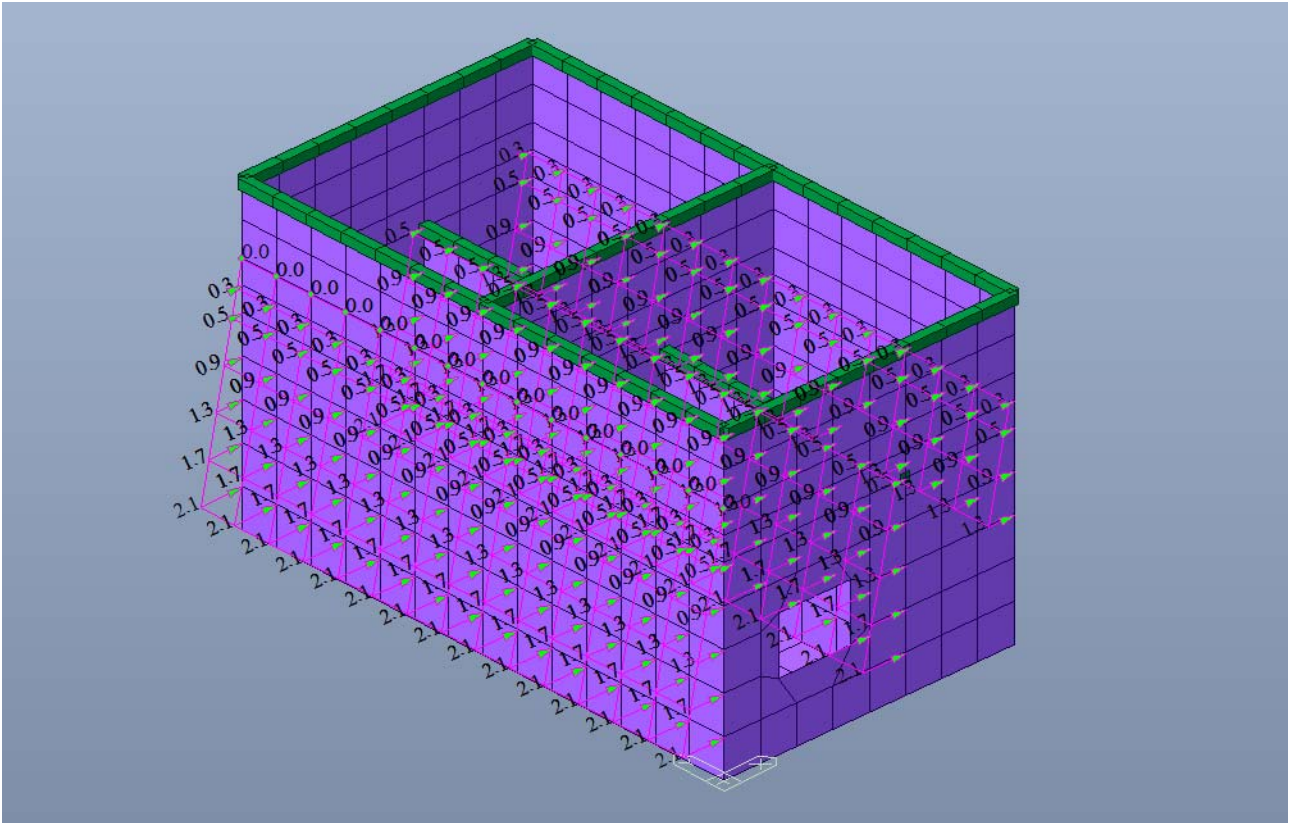
Assegnazione permanente magrone (PM) - [kN/mq]



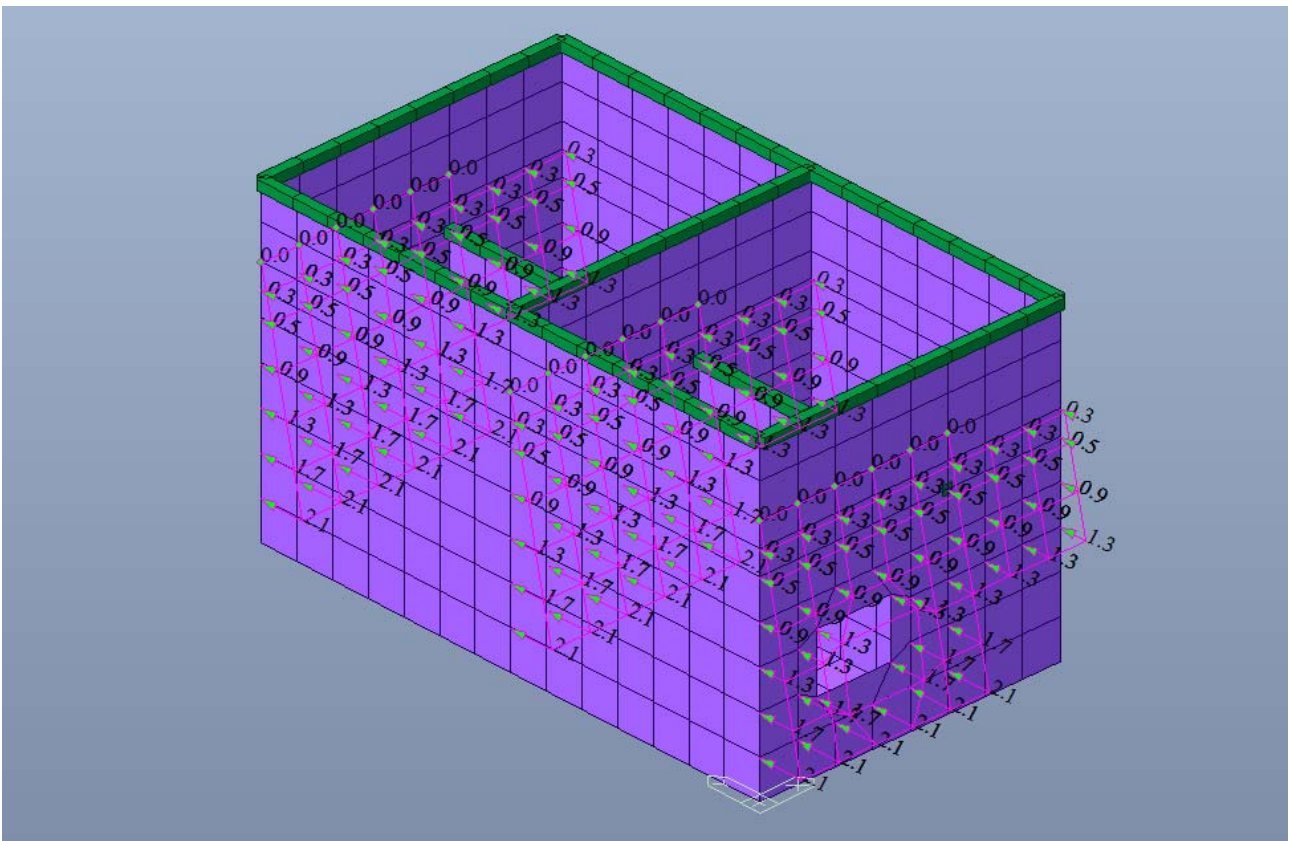
Assegnazione sovra spinta sismica terreno X (ET X) - [kN/mq]



Assegnazione sovra spinta sismica terreno Y (ET Y) - [kN/mq]



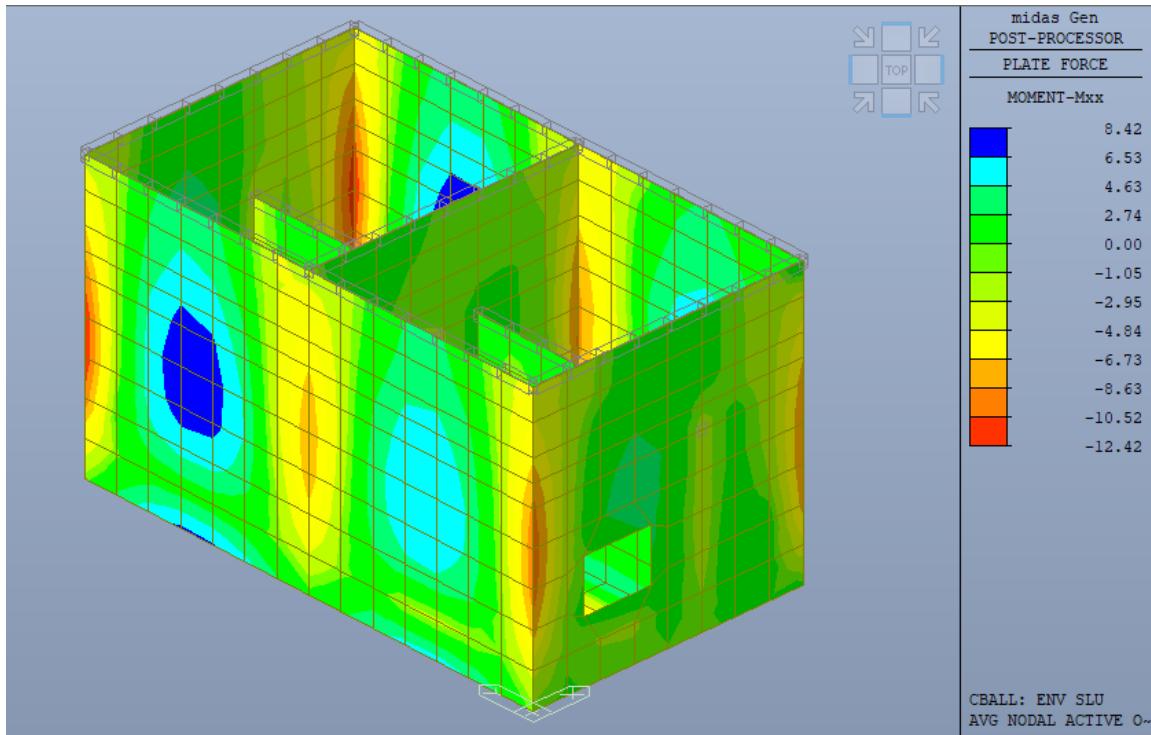
Assegnazione sovra spinta sismica liquame X (EL X) - [kN/mq]



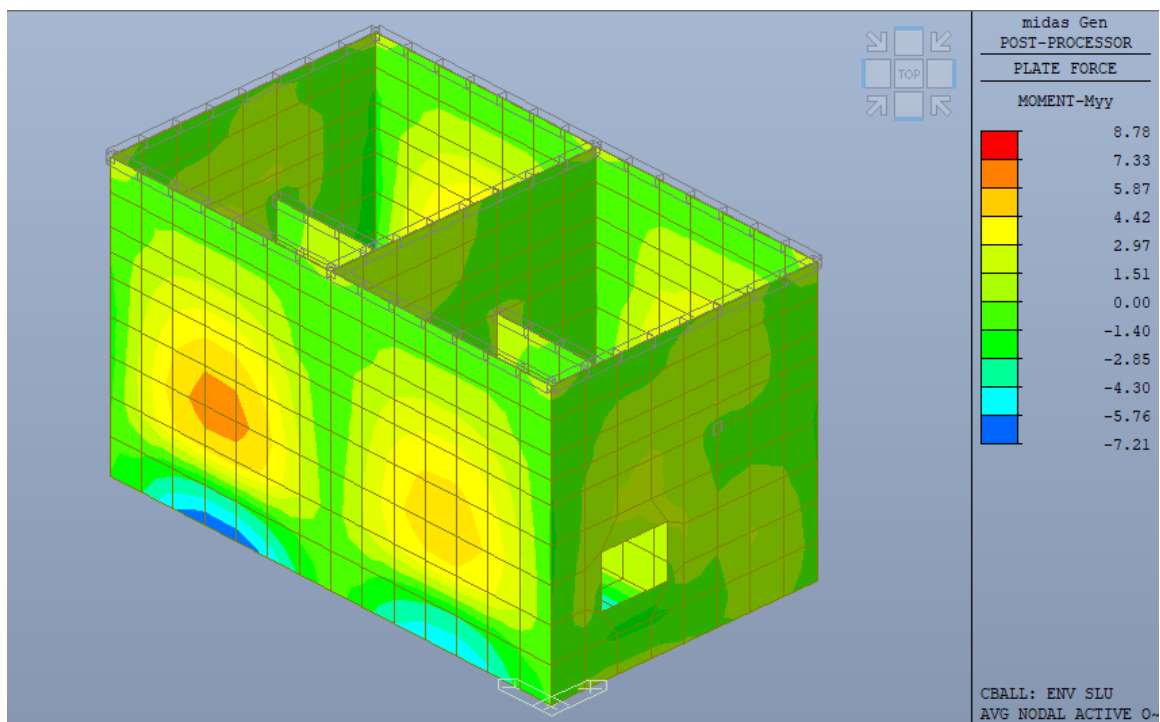
Assegnazione sovra spinta sismica liquame Y (EL Y) - [kN/mq]

1.3 Sollecitazioni

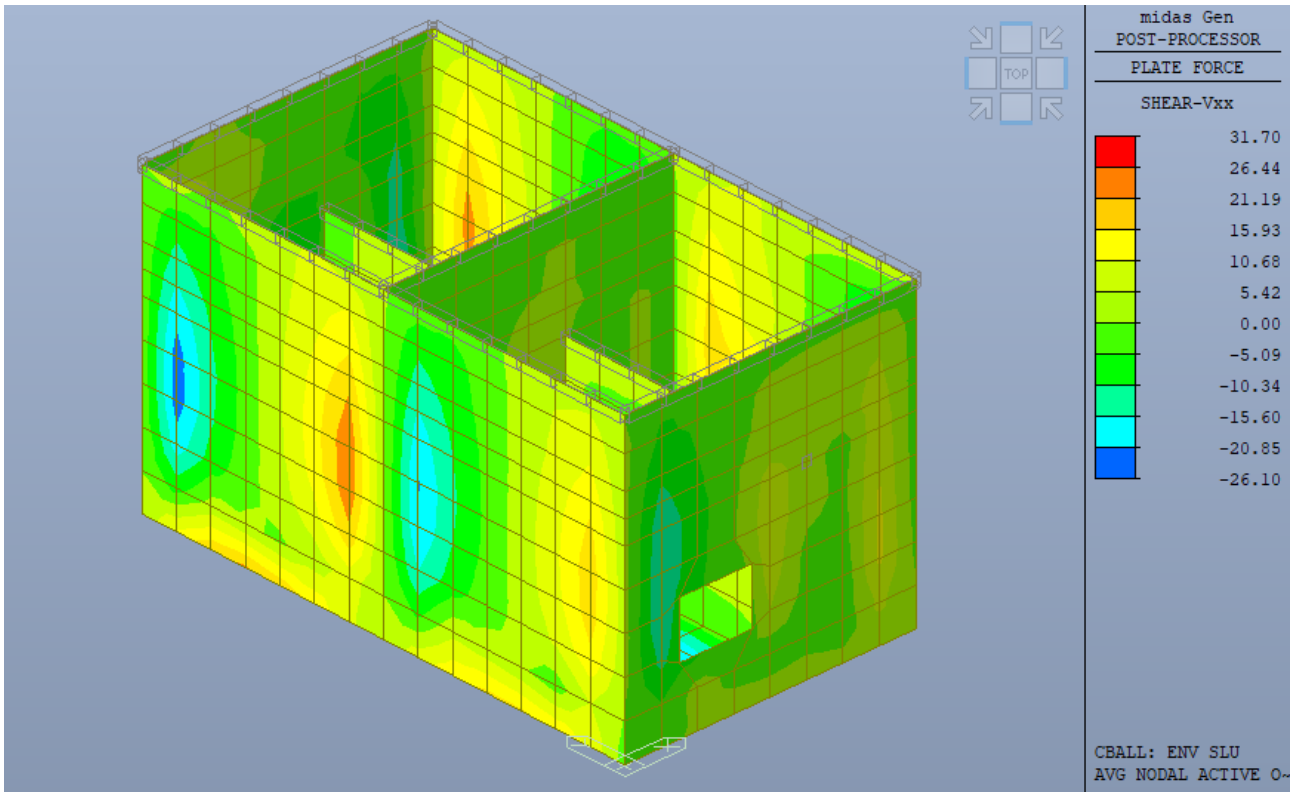
Nel presente paragrafo vengono riportate per via grafica le sollecitazioni sulla struttura indotte dai carichi applicati, per le varie combinazioni di carico statiche e sismiche.



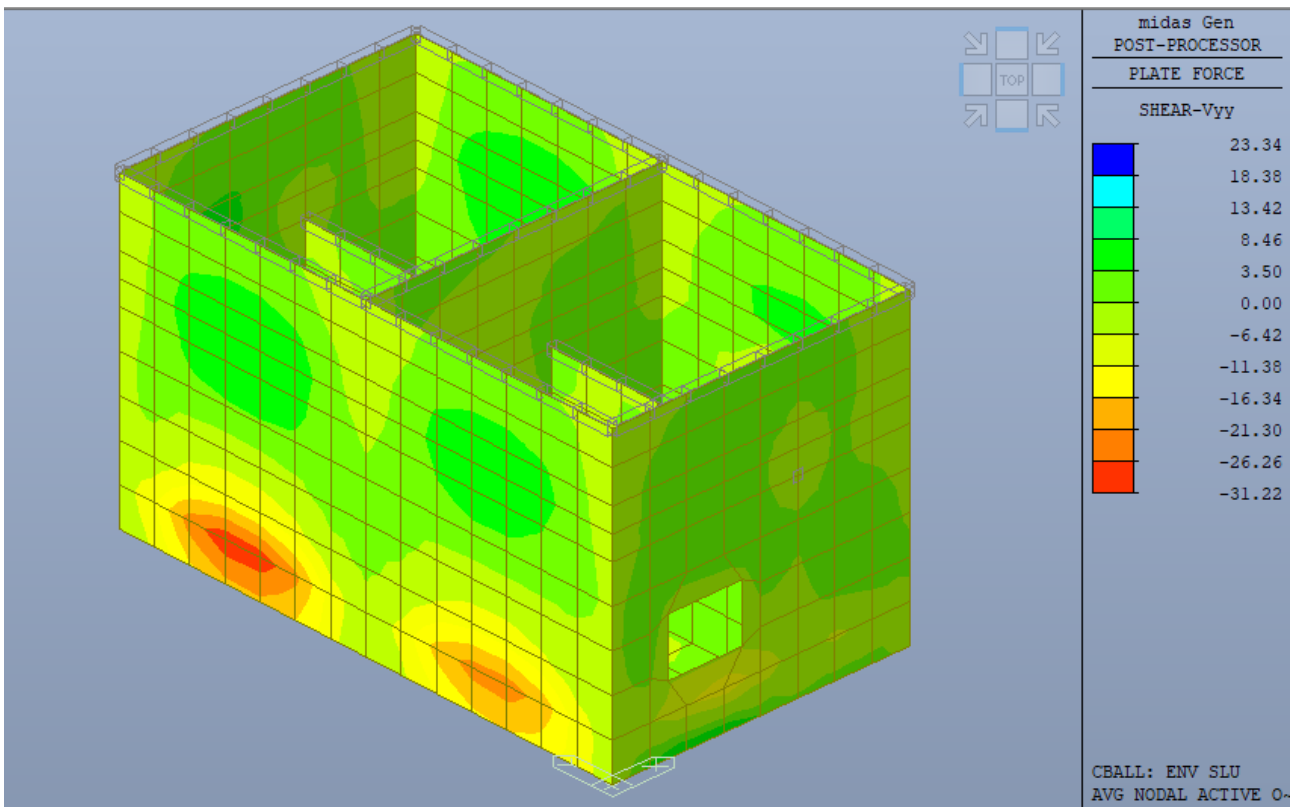
Momento flettente membranale M_{xx} – involucro SLU [kN*m/m]



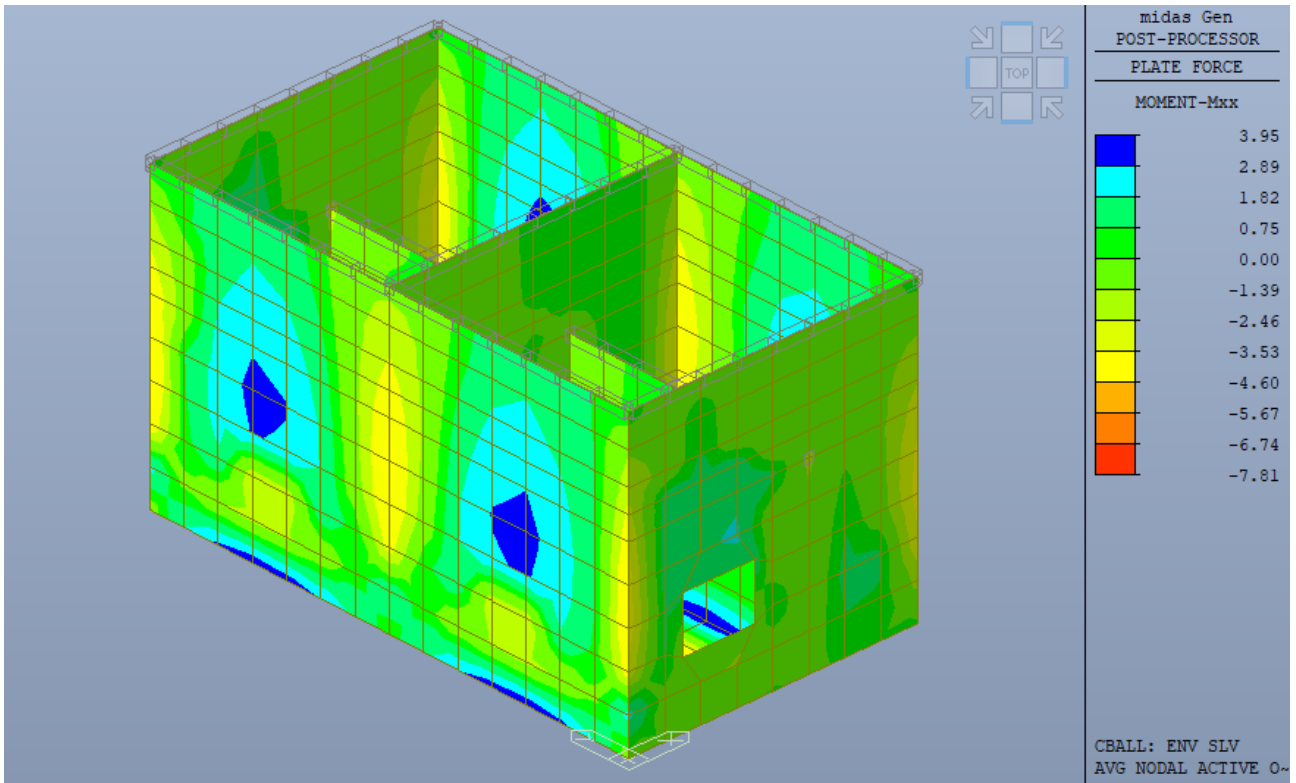
Momento flettente membranale M_{yy} – involucro SLU [kN*m/m]



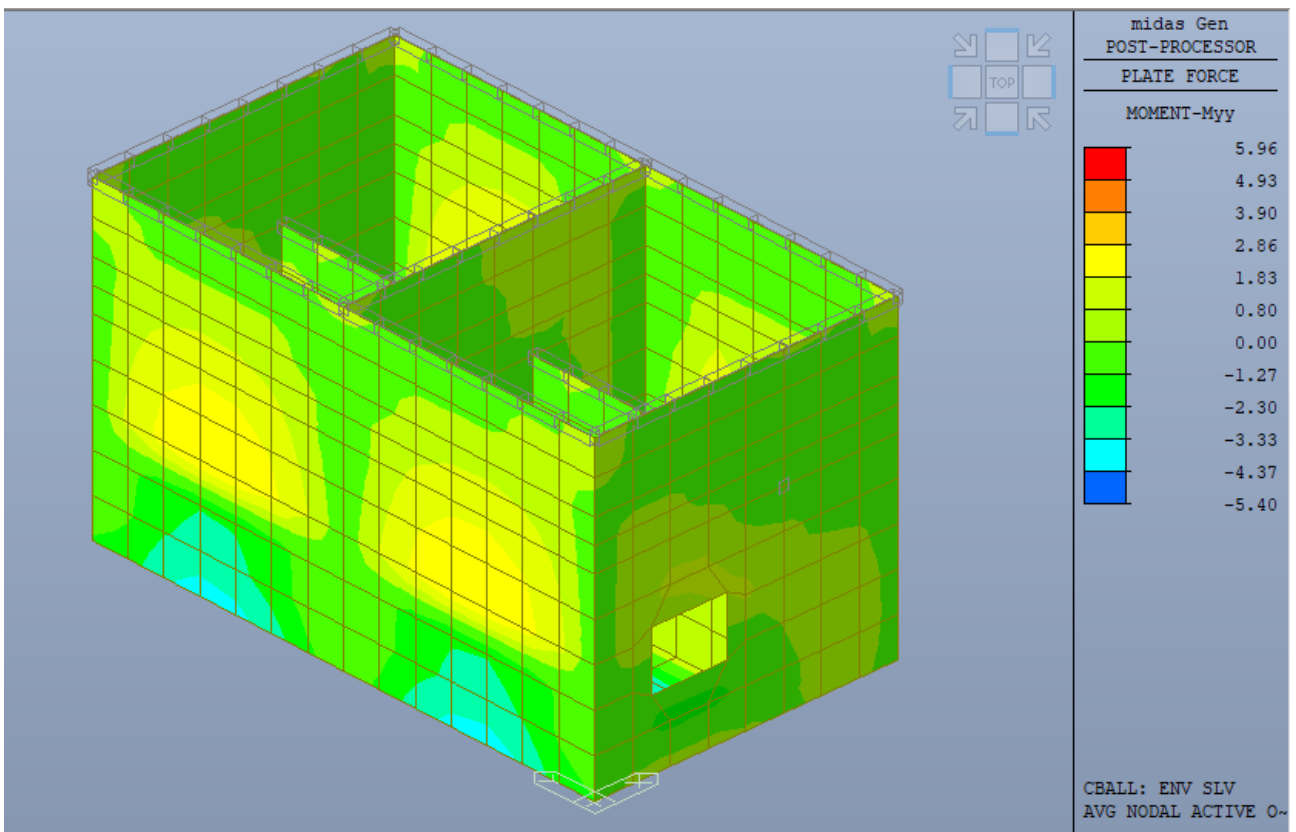
Sollecitazione tagliante Vxx – involucro SLU [kN/m]



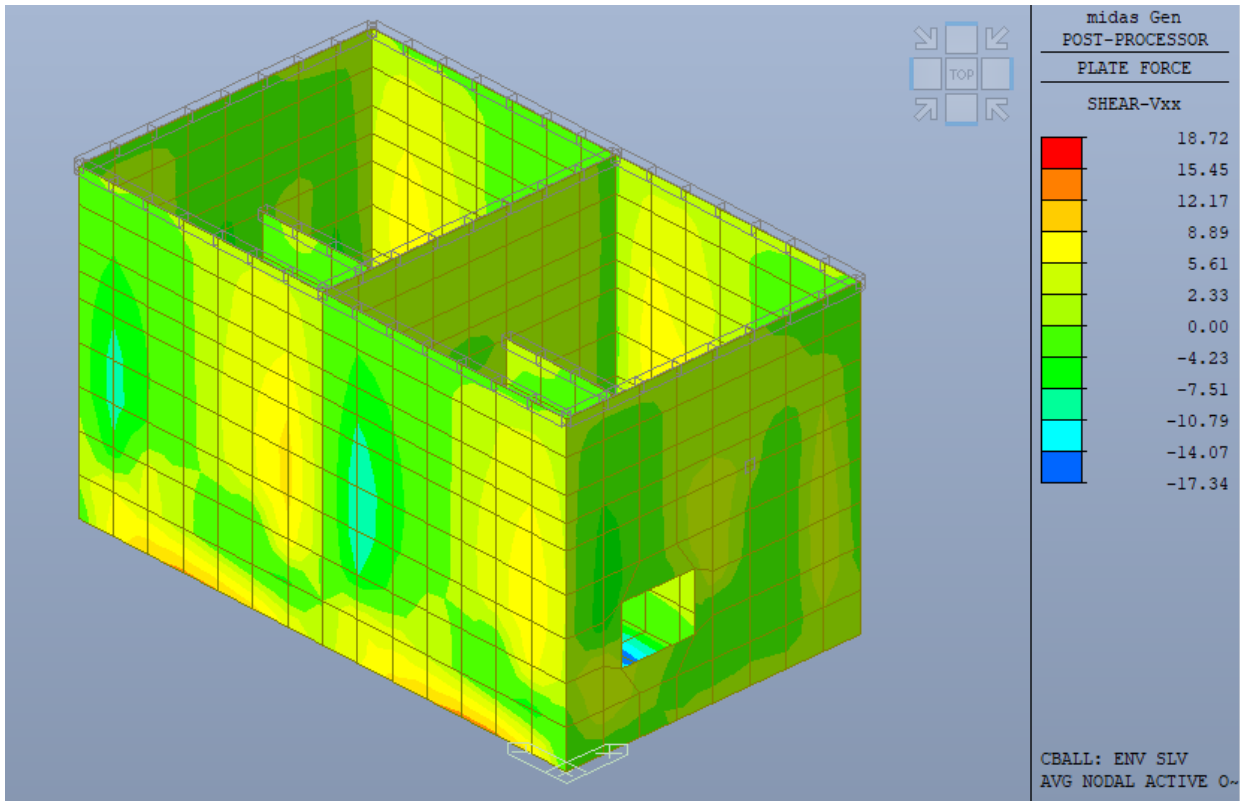
Sollecitazione tagliante Vyy – involucro SLU [kN/m]



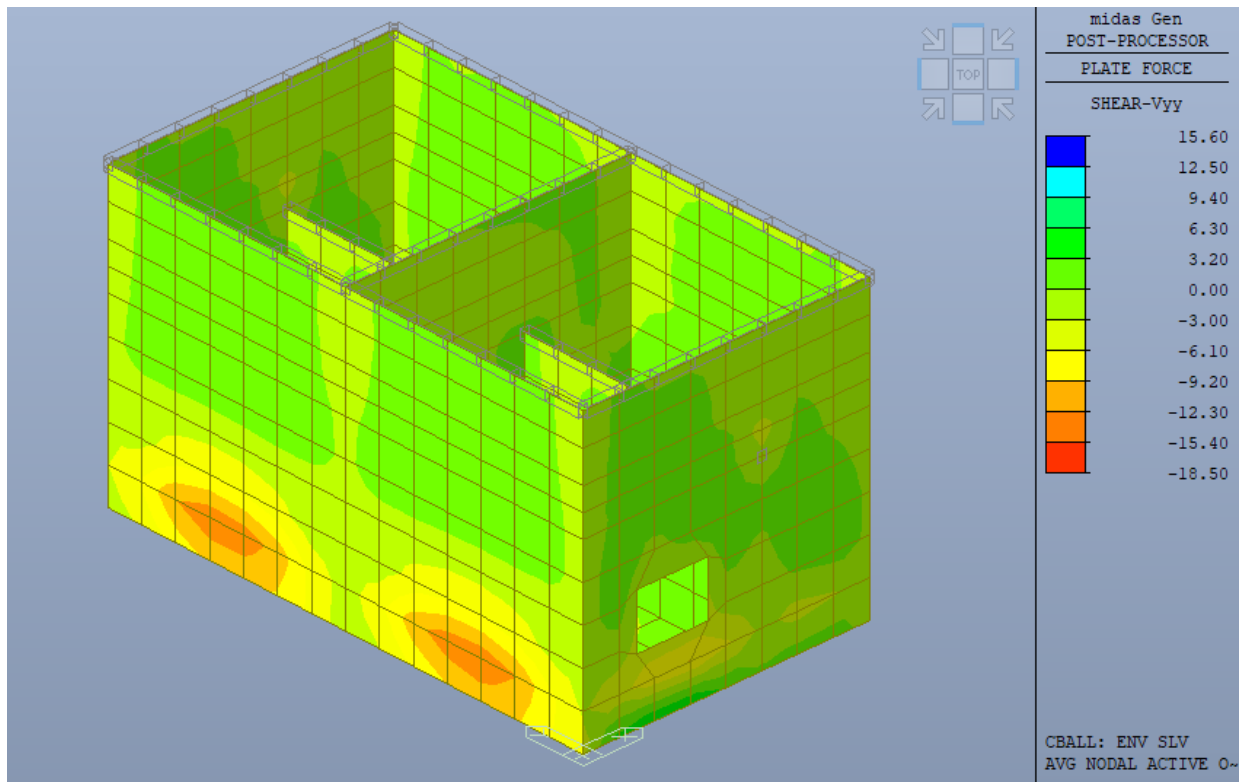
Momento flettente membranale M_{xx} – involucro SLV [kN*m/m]



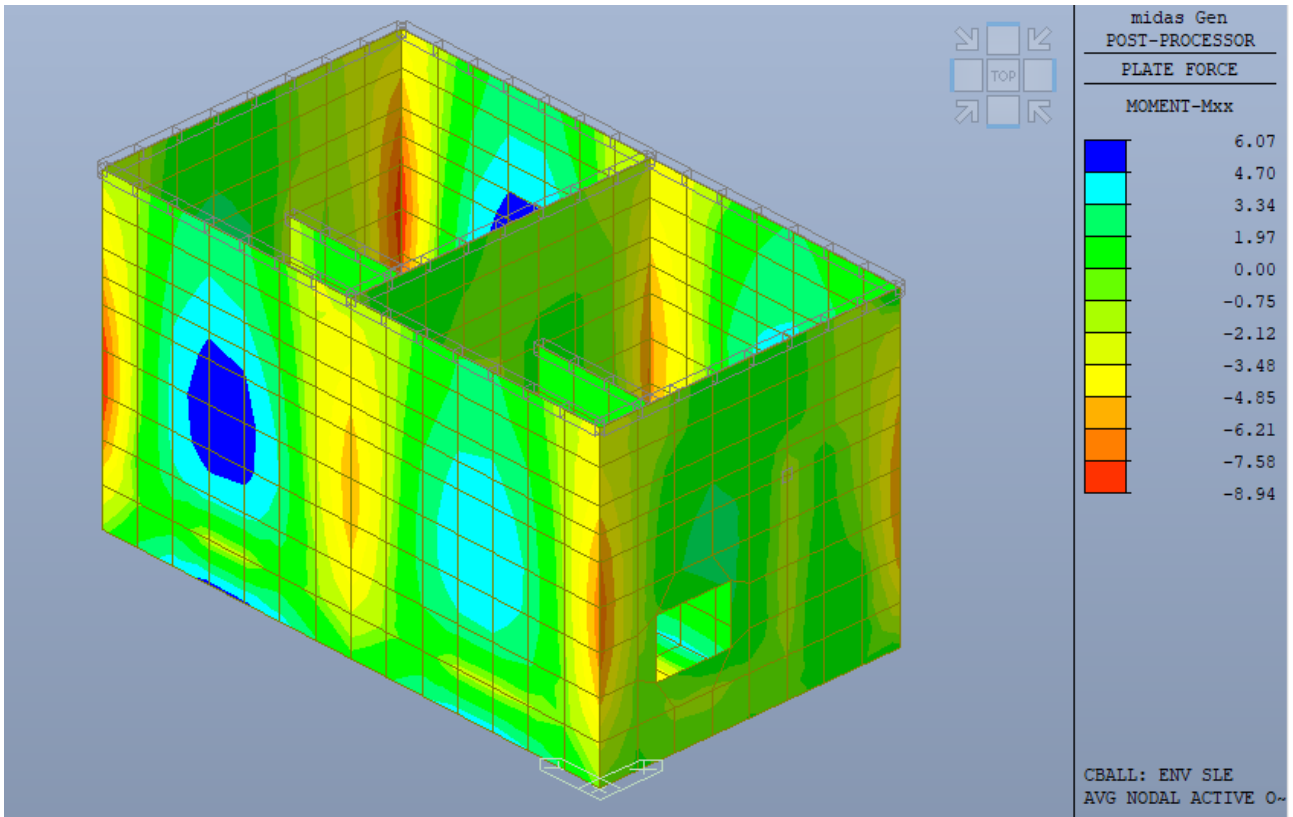
Momento flettente membranale M_{yy} – involucro SLV [kN*m/m]



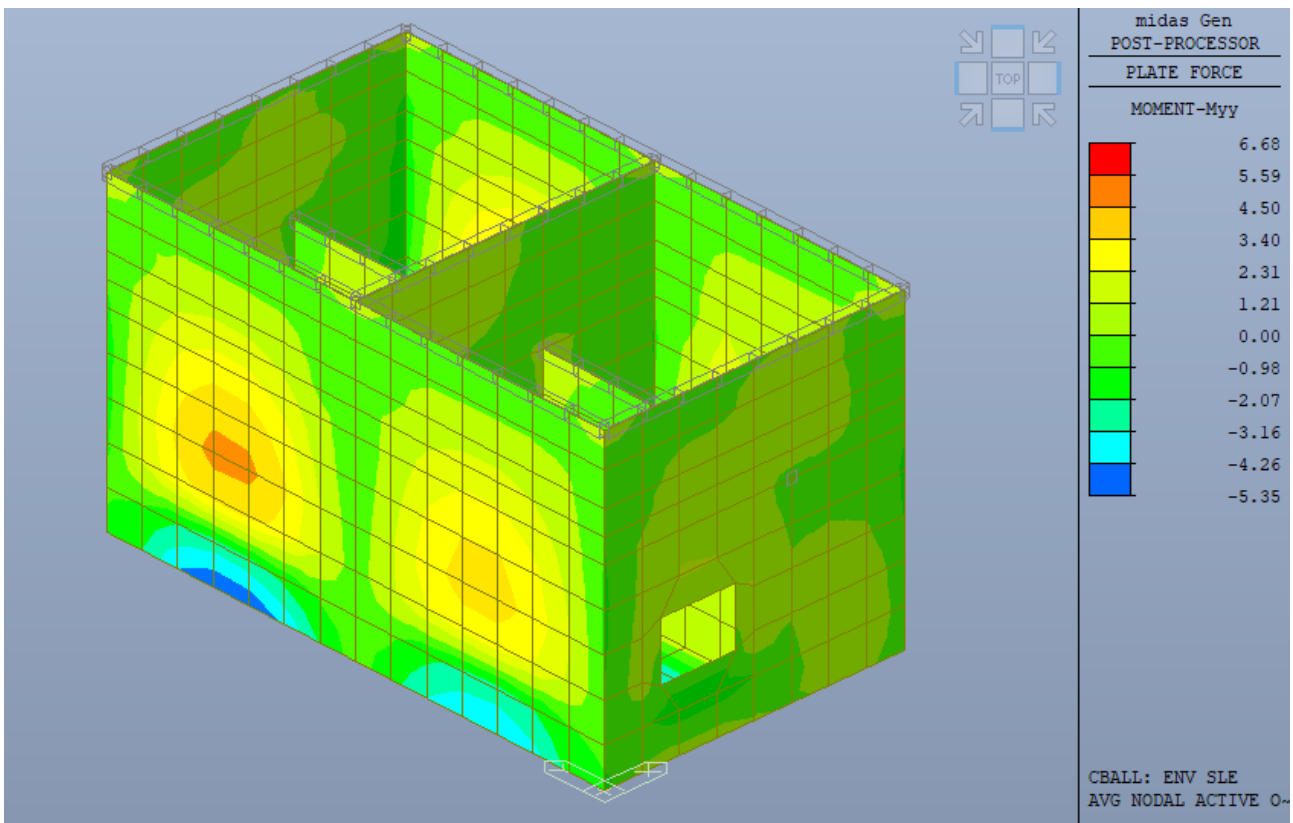
Sollecitazione tagliante Vxx – involucro SLV [kN/m]



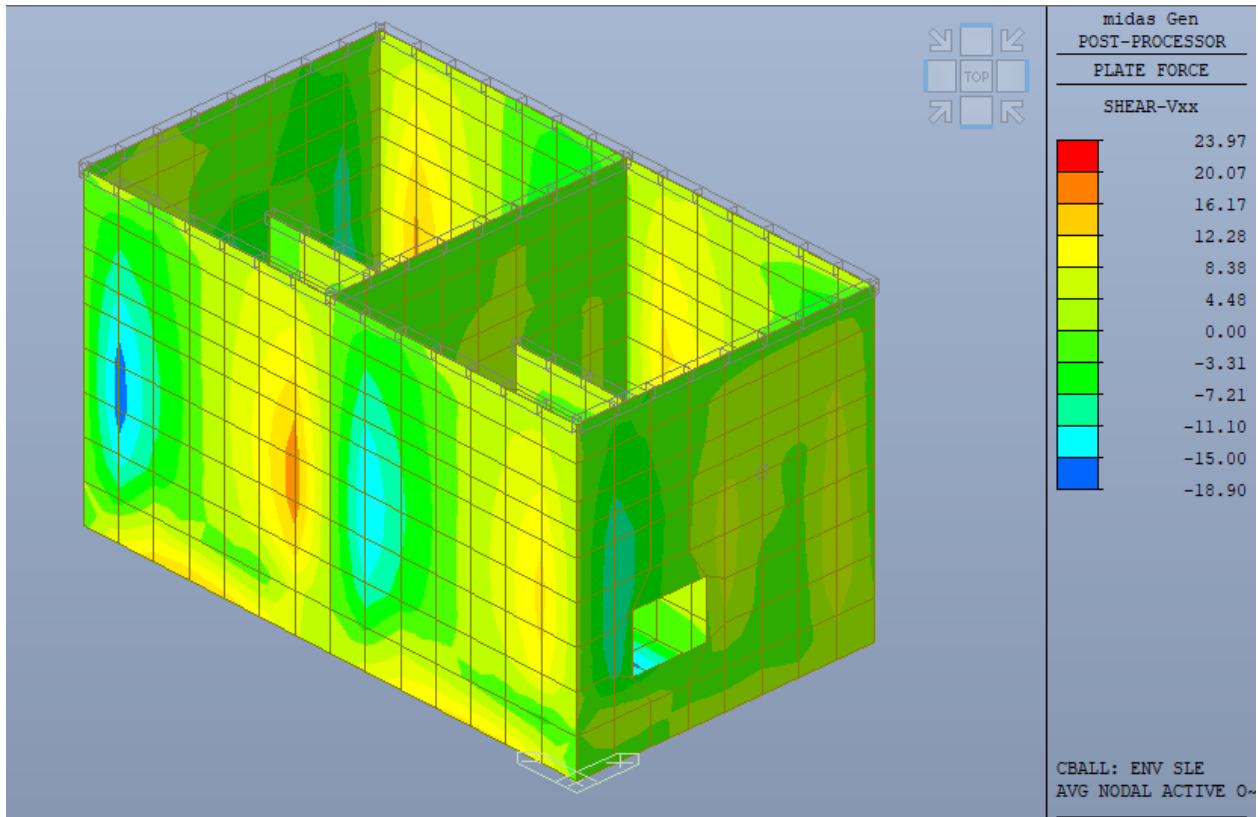
Sollecitazione tagliante Vyy – involucro SLV [kN/m]



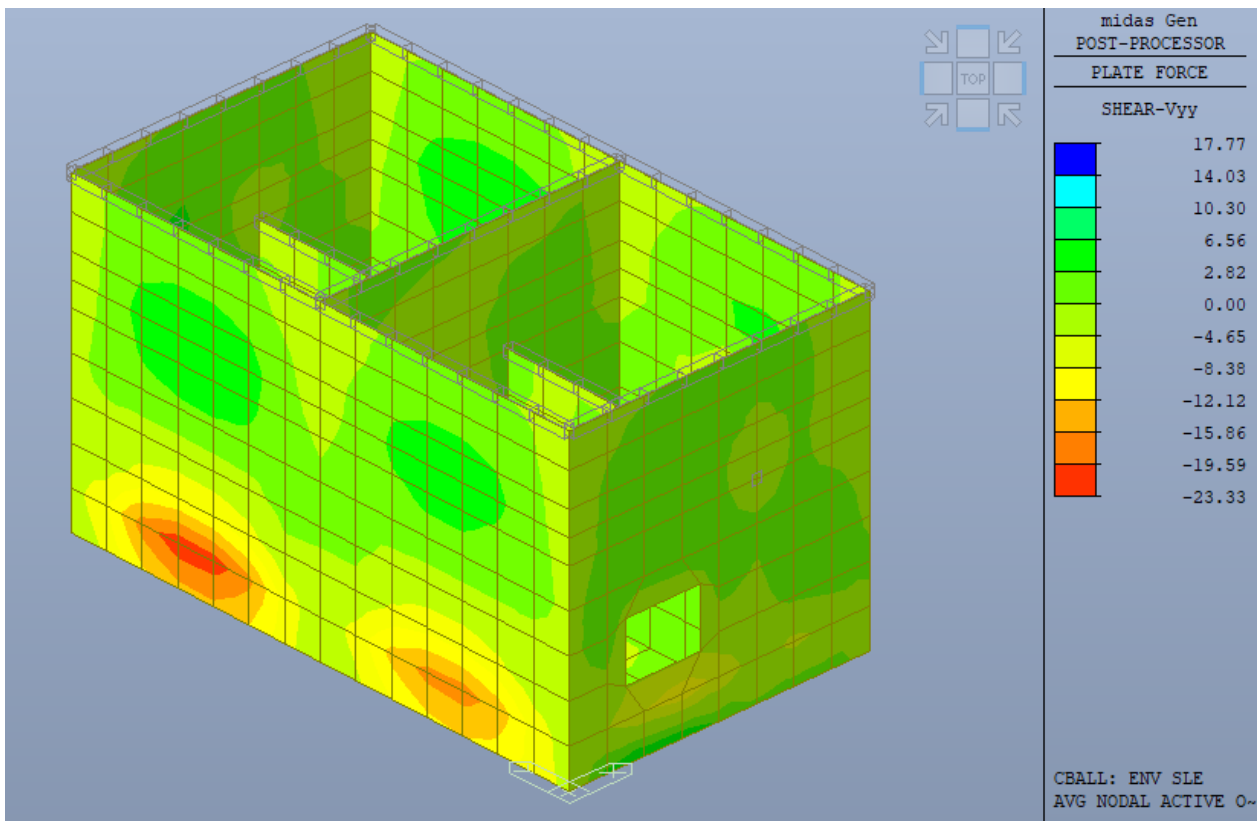
Momento flettente membranale Mxx – involucro SLE [kN*m/m]



Momento flettente membranale Myy – involucro SLE [kN*m/m]



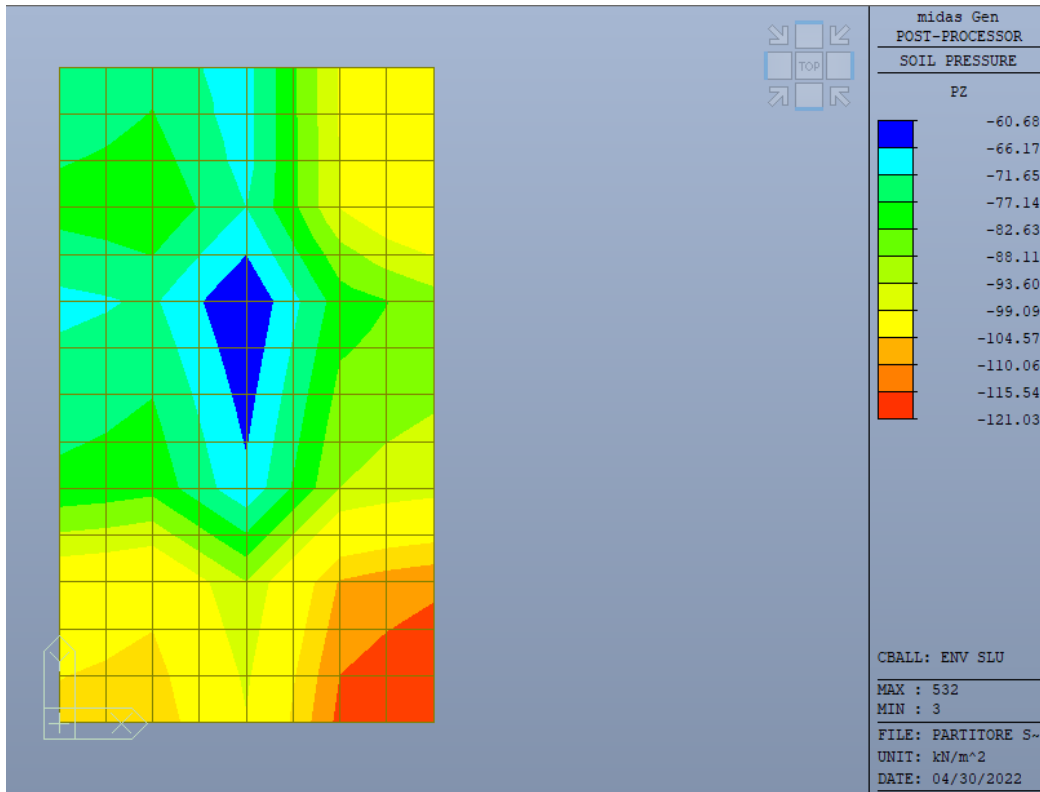
Sollecitazione tagliante Vxx – involucro SLE [kN/m]



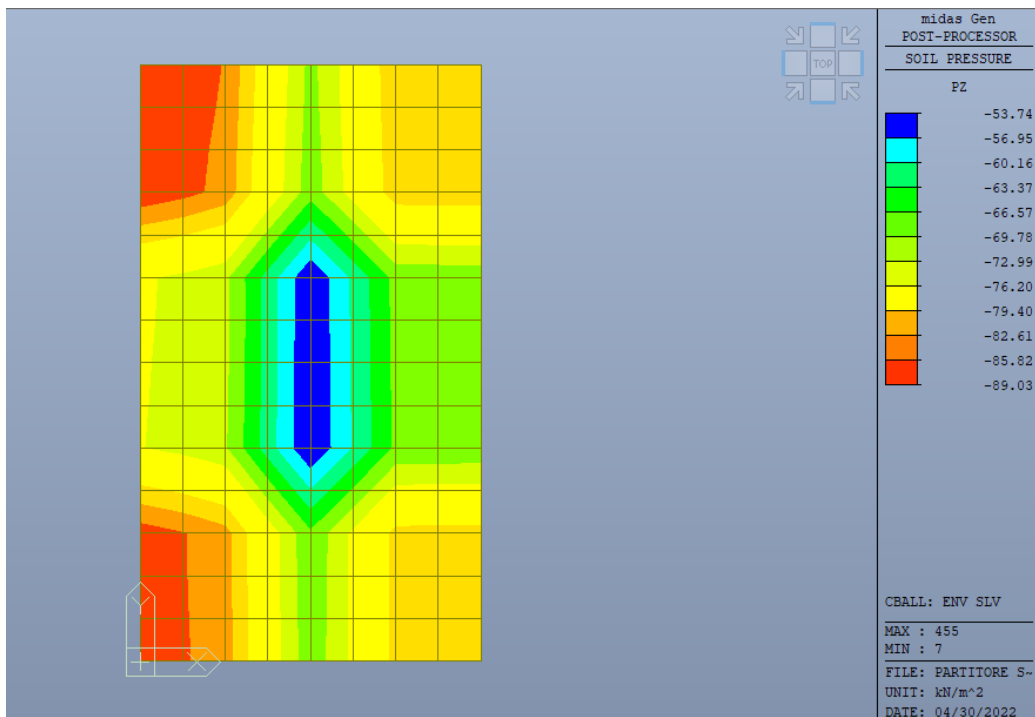
Sollecitazione tagliante Vyy – involucro SLE [kN/m]

1.4 Reazioni vincolari e pressioni sul terreno

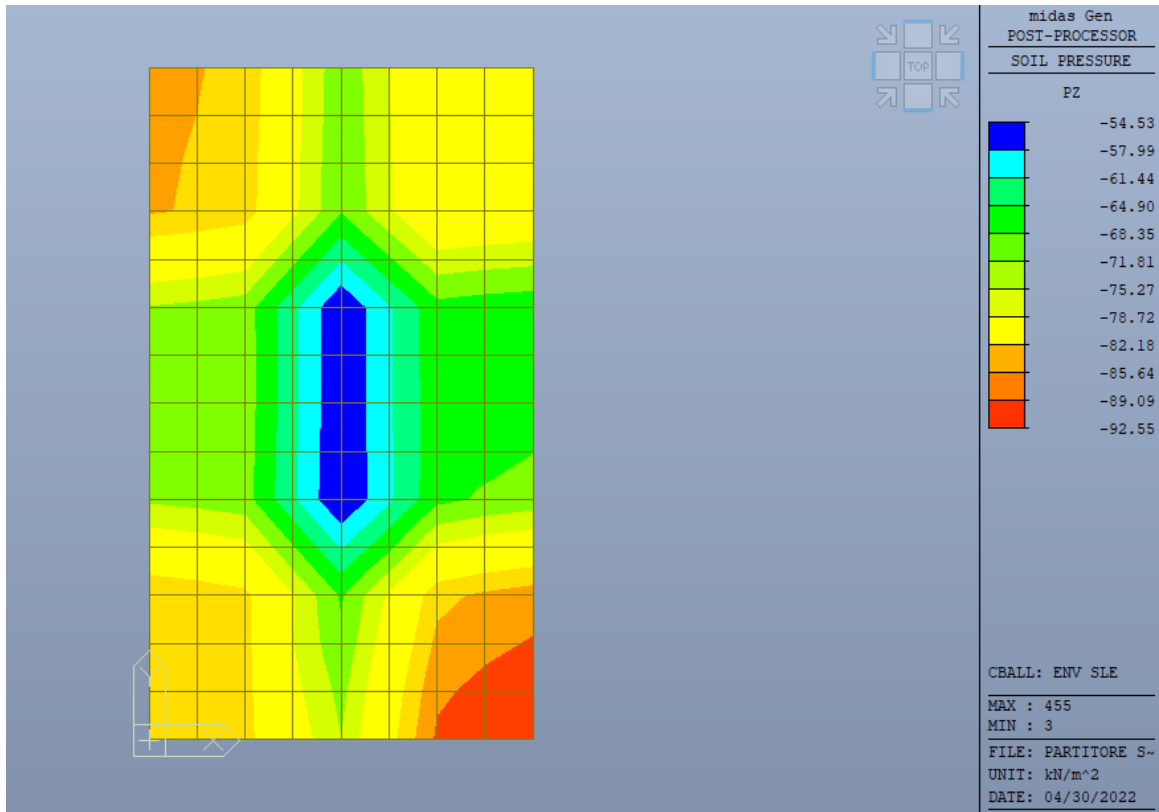
Nel presente paragrafo vengono riportate le reazioni vincolari e le pressioni sul terreno.



Pressioni sul terreno ENV SLU



Pressioni sul terreno ENV SLV



Pressioni sul terreno ENV SLE

SUMMATION OF REACTION FORCES PRINTOUT						
	Load	FX (kN)	FY (kN)	FZ (kN)		
	SLU 1	-10.253568	2.957760	1319.756000		
	SLU 2	-14.926080	4.305600	1086.796000		
	SLU 3	59.852182	-7.338240	1527.964000		
	SLU 4	-19.853568	5.357760	1249.196000		
	SLU 5	53.132182	-5.658240	1527.964000		
	SLU 6	55.179670	-5.990400	1527.964000		
	SLU 7	48.459670	-4.310400	1527.964000		
	SLU 8	45.579670	-3.590400	1506.796000		
	SLV 1	-10.389760	2.462880	960.920000		
	SLV 2	-10.389760	2.087520	960.920000		
	SLV 3	-5.384960	2.462880	960.920000		
	SLV 4	-5.384960	2.087520	960.920000		
	SLV 5	-8.638080	2.900800	960.920000		
	SLV 6	-8.638080	1.649600	960.920000		
	SLV 7	-7.136640	2.900800	960.920000		
	SLV 8	-7.136640	1.649600	960.920000		
	SLV 9	66.272240	-7.376970	1281.240000		
	SLV 10	66.272240	1.367370	1281.240000		
	SLV 11	127.679040	-7.376970	1281.240000		
	SLV 12	127.679040	1.367370	1281.240000		
	SLV 13	87.764620	-17.578700	1281.240000		
	SLV 14	87.764620	11.569100	1281.240000		
	SLV 15	106.186660	-17.578700	1281.240000		
	SLV 16	106.186660	11.569100	1281.240000		
	SLE R1	-7.887360	2.275200	1007.960000		
	SLE R2	-11.481600	3.312000	828.760000		
	SLE R3	46.040140	-5.644800	1168.120000		
	SLE R4	-14.287360	3.875200	960.920000		
	SLE R5	41.560140	-4.524800	1168.120000		
	SLE R6	42.445900	-4.608000	1168.120000		
	SLE R7	37.965900	-3.488000	1168.120000		
	SLE R8	36.045900	-3.008000	1144.600000		
	SLE F1	-7.887360	2.275200	984.440000		
	SLE F2	-9.807360	2.755200	984.440000		
	SLE F3	96.975640	-3.004800	1304.760000		
	SLE F4	95.055640	-2.524800	1304.760000		
	SLE Qp	96.975640	-3.004800	1281.240000		

Reazioni vincolari

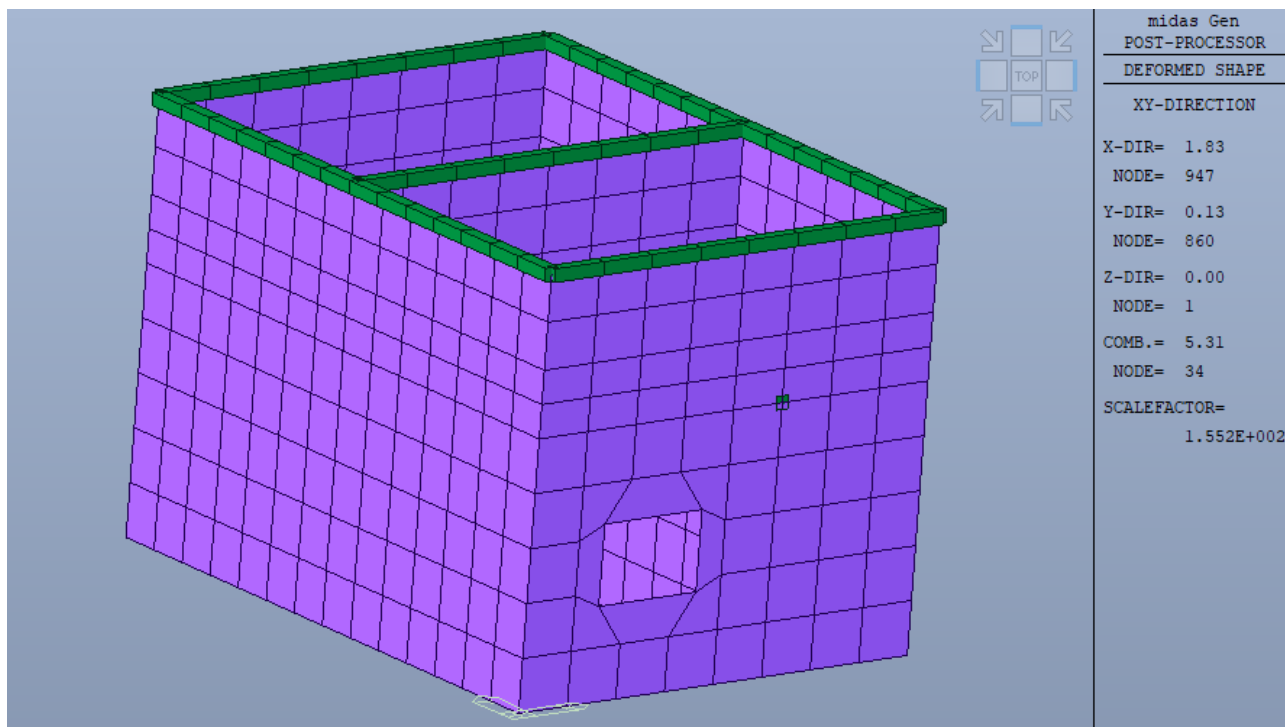
1.5 Deformazioni

Per le costruzioni ricadenti in classe d'uso IV si deve verificare che l'azione sismica di progetto non produca danni agli elementi costruttivi senza funzione strutturale tali da rendere temporaneamente non operativa la costruzione (rif §7.3.6.1 NTC 2018)

Nel caso delle costruzioni civili e industriali questa condizione si può ritenere soddisfatta quando gli spostamenti interpiano ottenuti dall'analisi in presenza dell'azione sismica di progetto relativa allo SLO siano inferiori ai 2/3 dei seguenti limiti:

- per tamponamenti collegati rigidamente alla struttura che interferiscono con la deformabilità della stessa: $dr < 0,005 h$
- per tamponamenti progettati in modo da non subire danni a seguito di spostamenti di interpiano dr_p , per effetto della loro deformabilità intrinseca ovvero dei collegamenti alla struttura: $dr \leq dr_p \leq 0,01 h$
- per costruzioni con struttura portante in muratura ordinaria: $dr < 0,003 h$
- per costruzioni con struttura portante in muratura armata: $dr < 0,004 h$

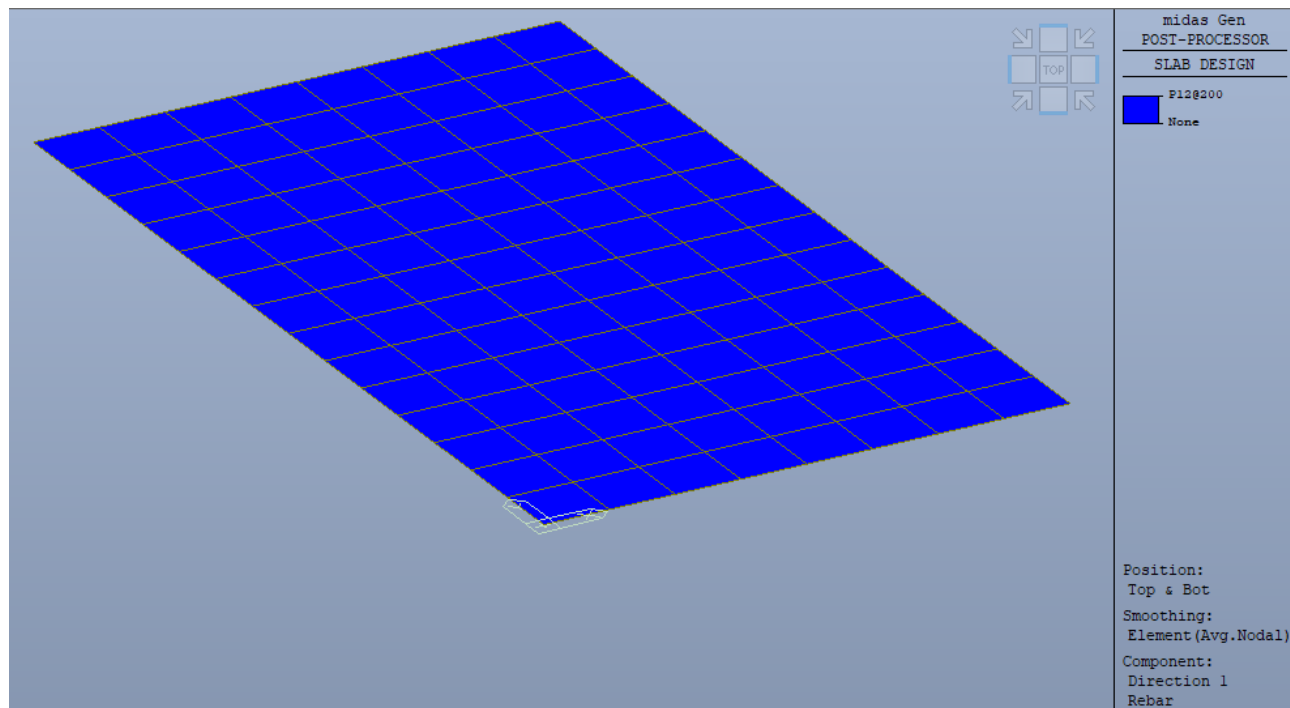
Premesso che l'opera in oggetto è completamente in c.a. e che quindi anche il superamento di questi limiti non produrrebbe alcun effetto, dall'immagine seguente si evince che lo spostamento massimo sotto l'azione sismica a SLO è pari a 1.8 mm che corrisponde a circa 0,0004 h, quindi trascurabile.



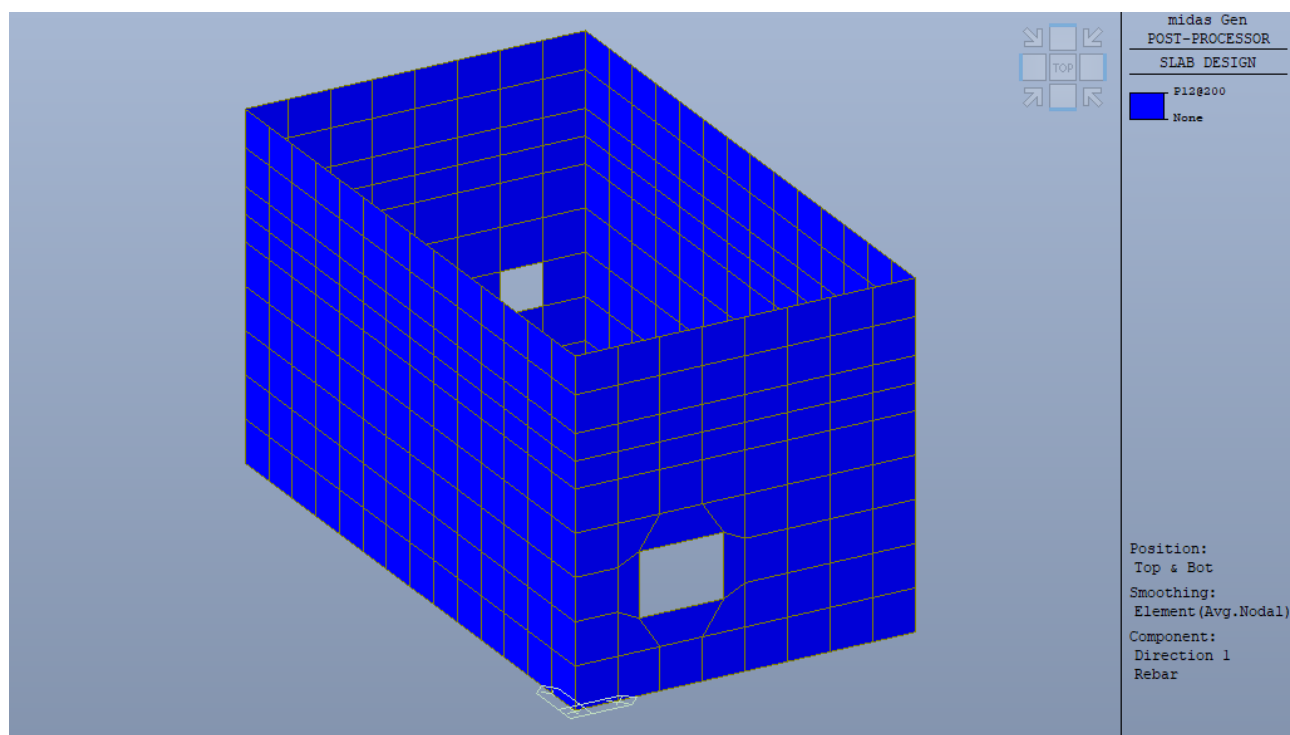
Massime deformazioni SLO combinata XY [cm]

1.6 Armature previste

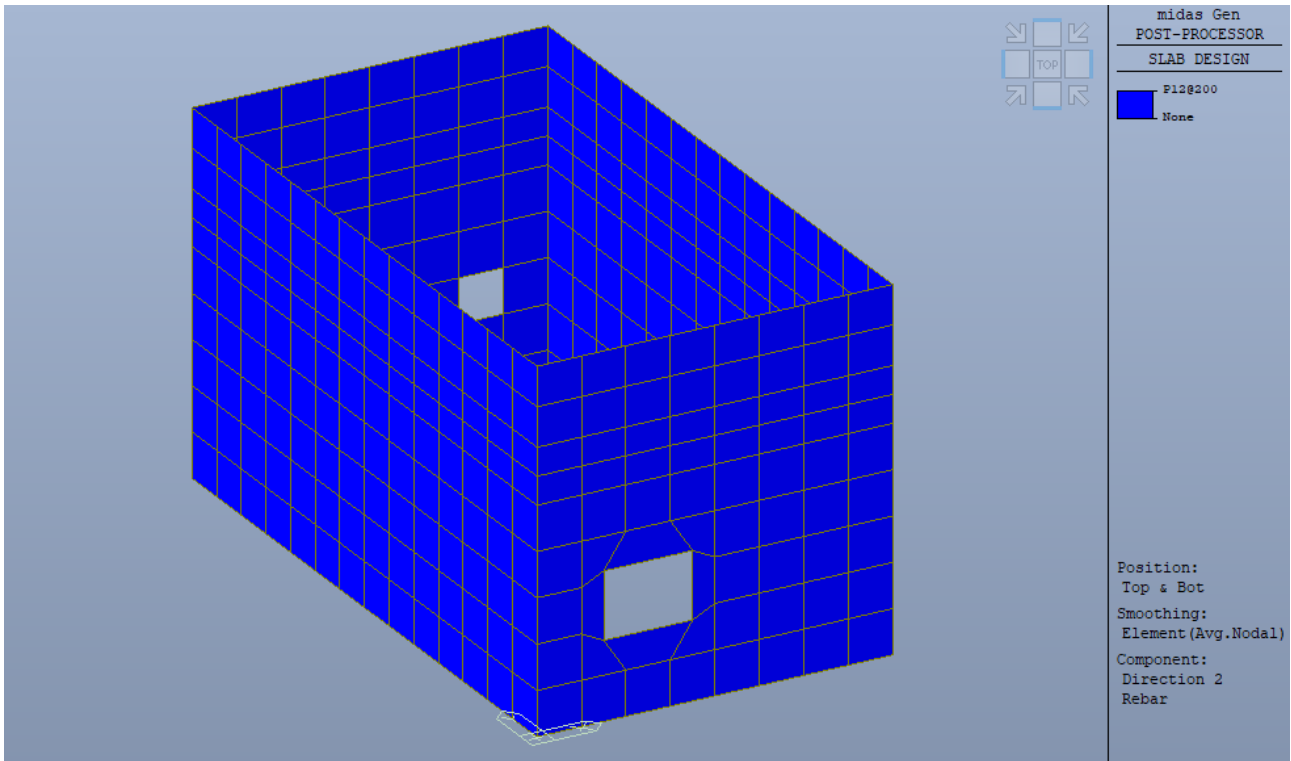
Nelle immagini seguenti vengono riportate le armature previste per i vari elementi strutturali.



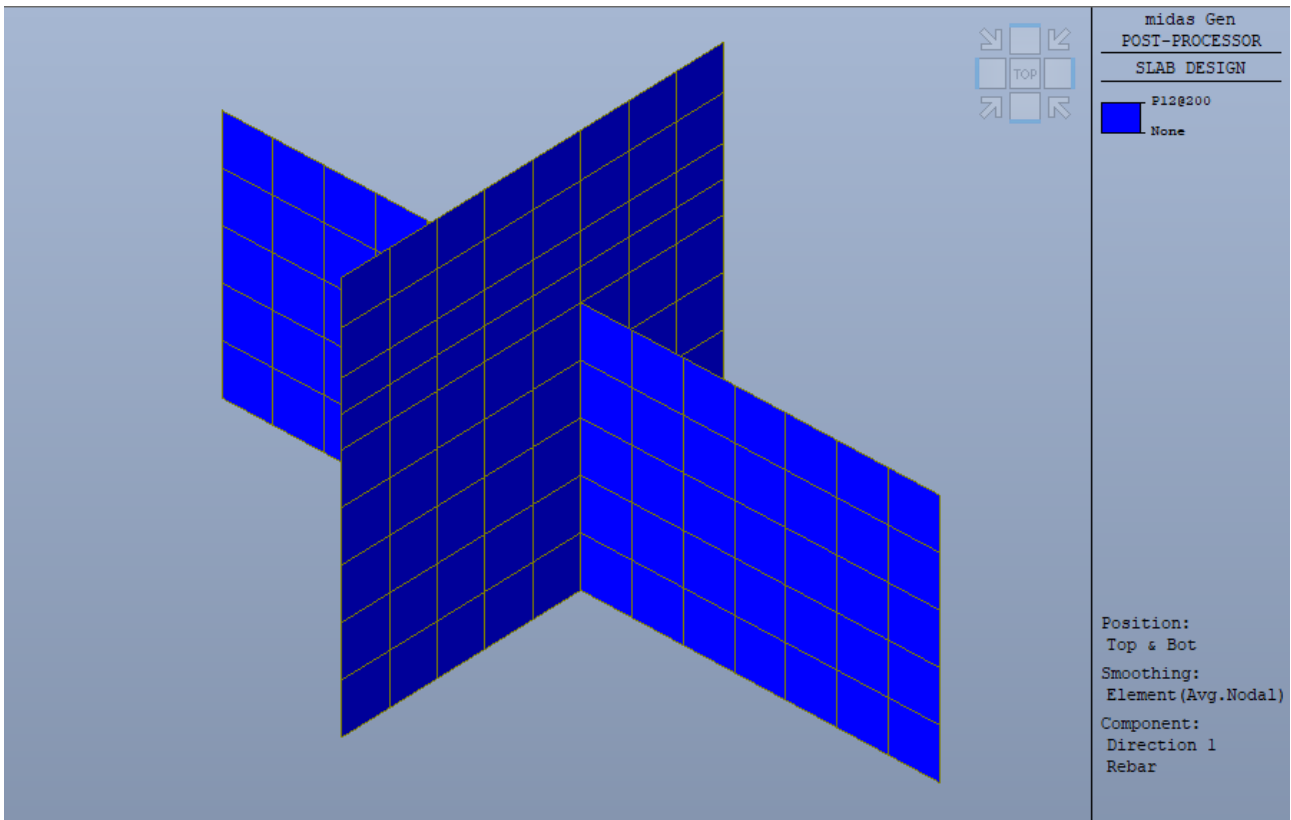
Platea – armature in direzione X e Y – ambo i lati



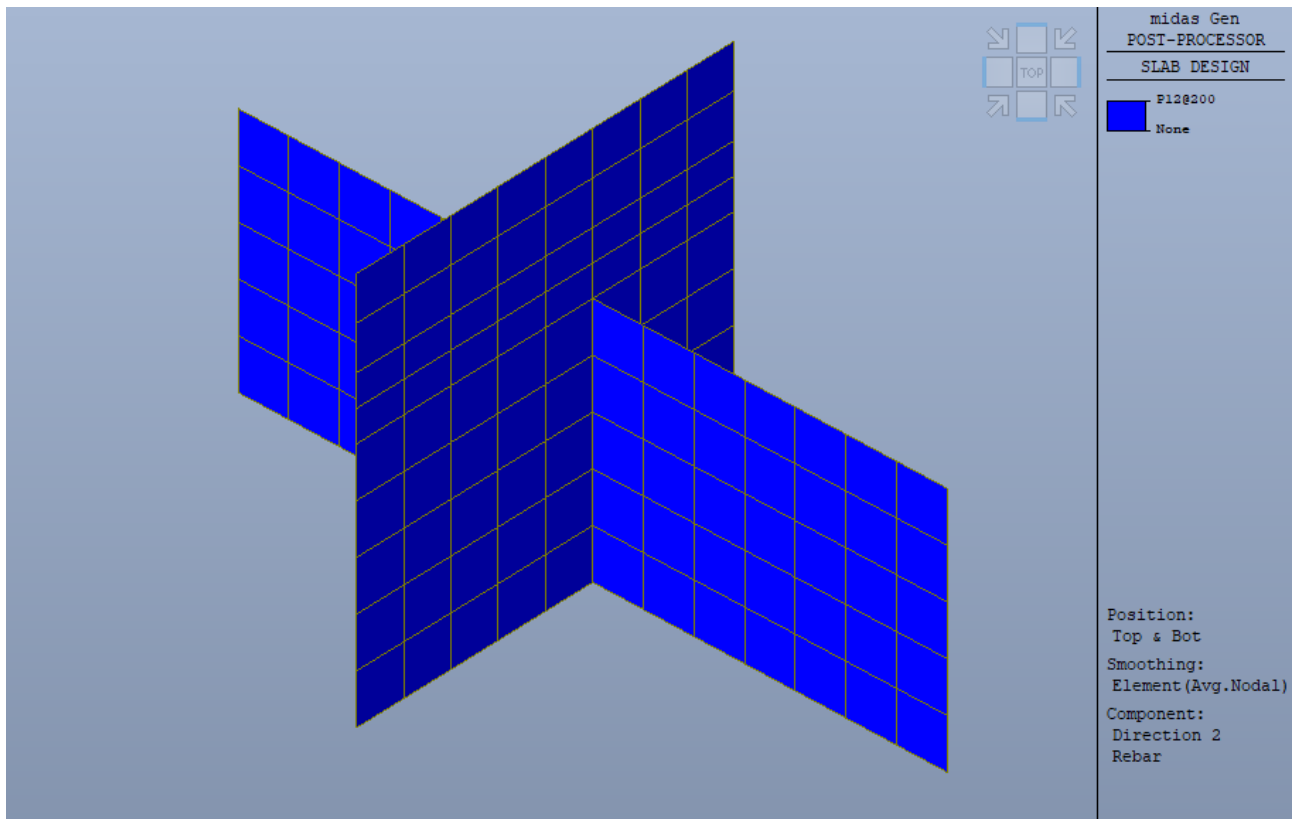
Pareti esterne – armature in direzione orizzontale – ambo i lati



Pareti esterne – armature in direzione verticale – ambo i lati



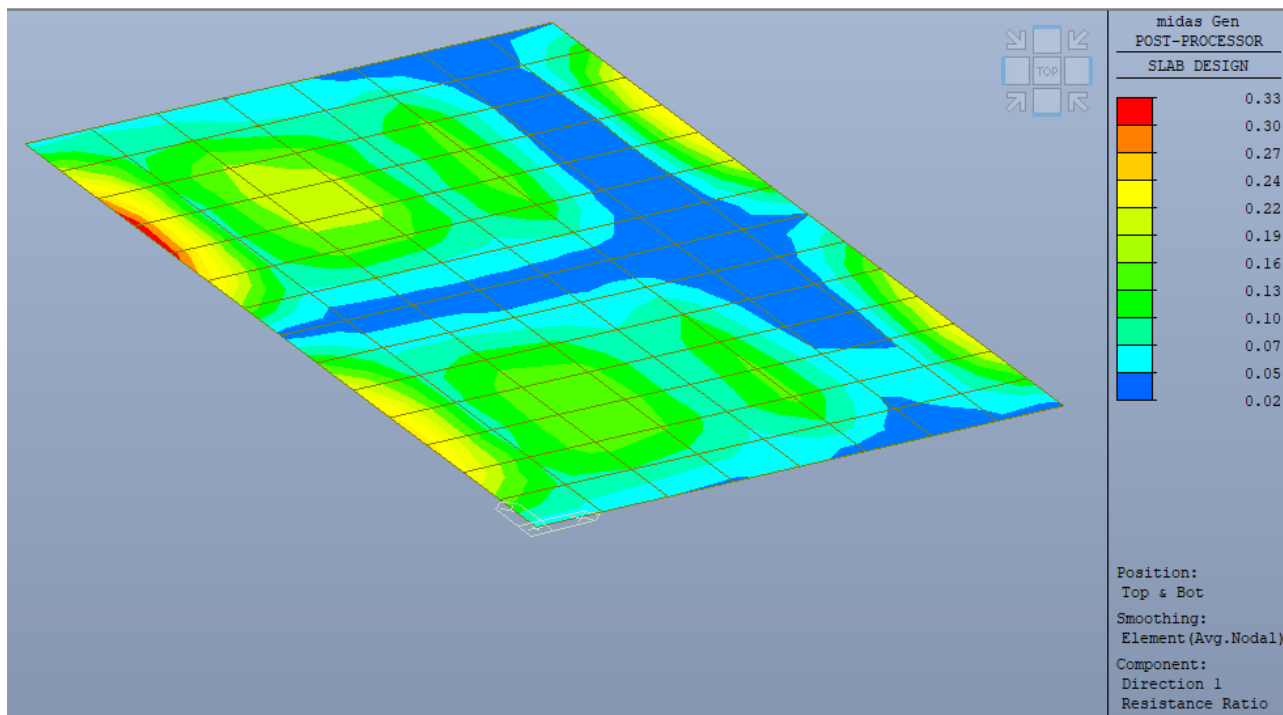
Pareti interne – armature in direzione orizzontale – ambo i lati



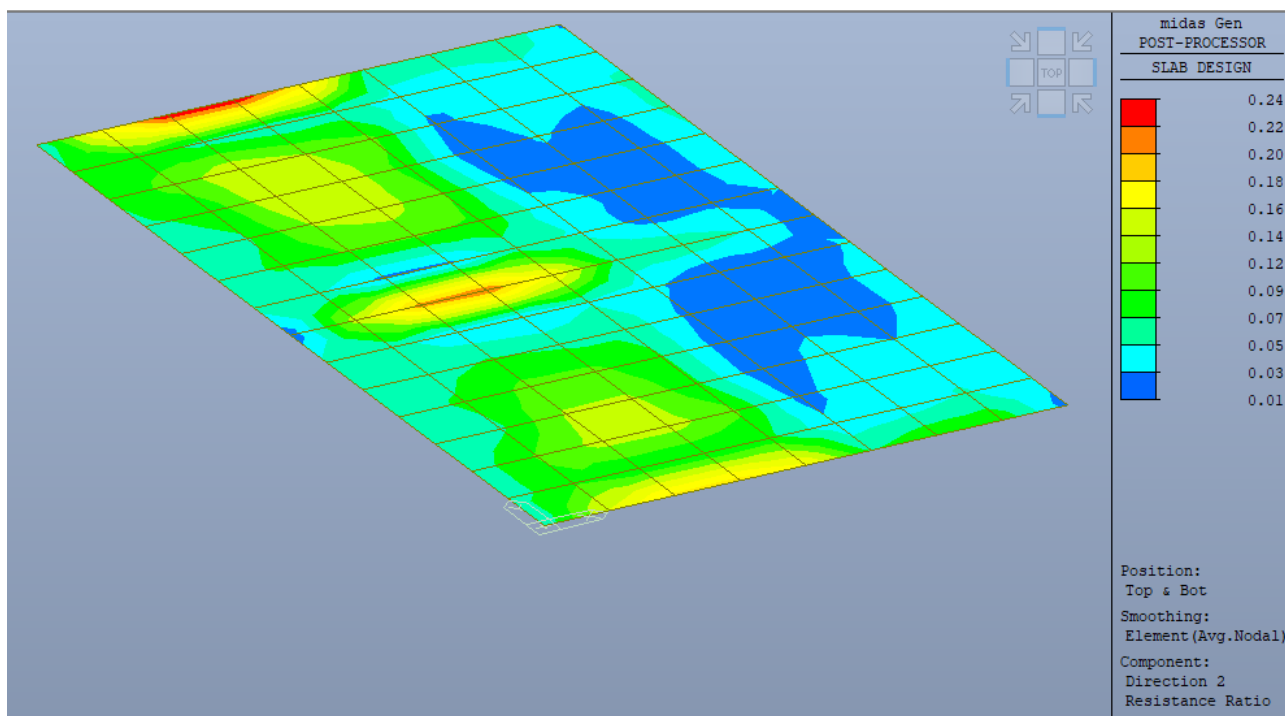
Pareti interne – armature in direzione verticale – ambo i lati

1.7 Verifiche di resistenza SLU grafiche

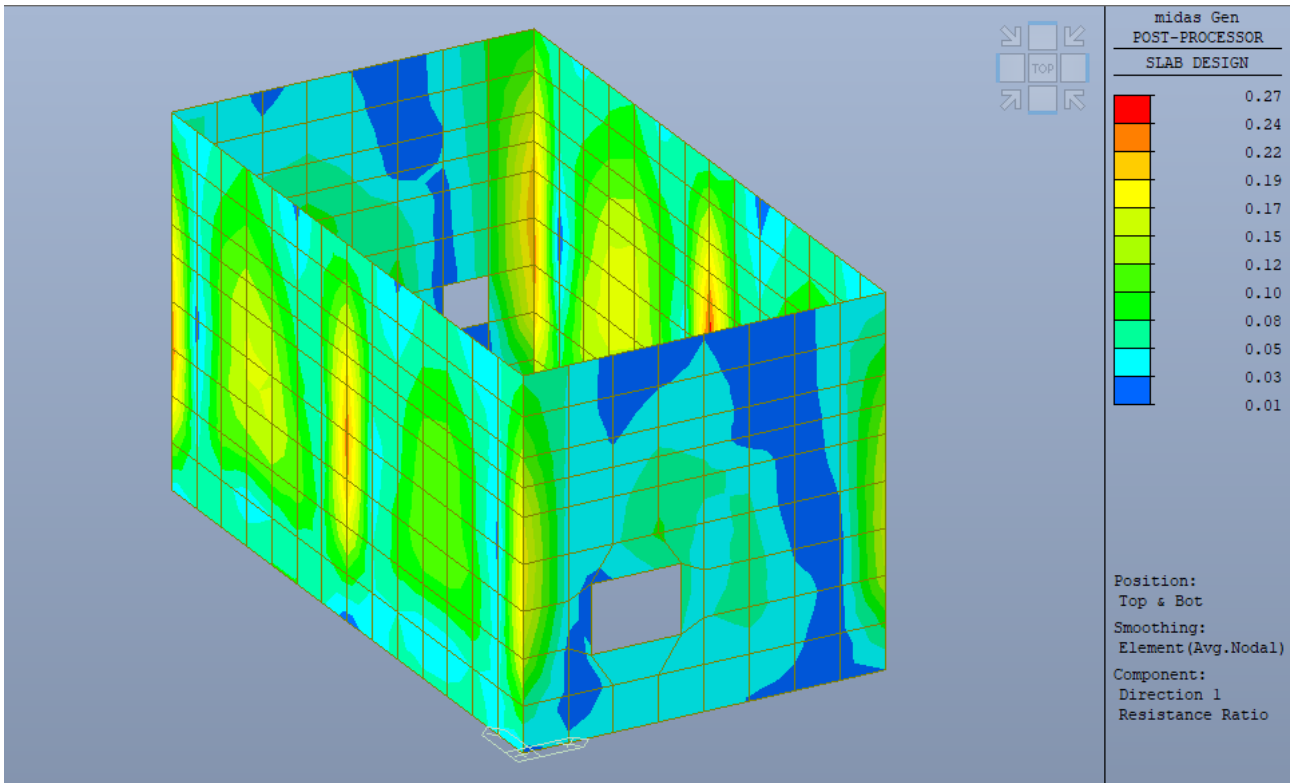
Nelle immagini seguenti vengono riportate le verifiche strutturali per via grafica, come tassi di sfruttamento dell'armatura nelle sezioni di cemento armato, sia per le sollecitazioni flessionali che taglianti:



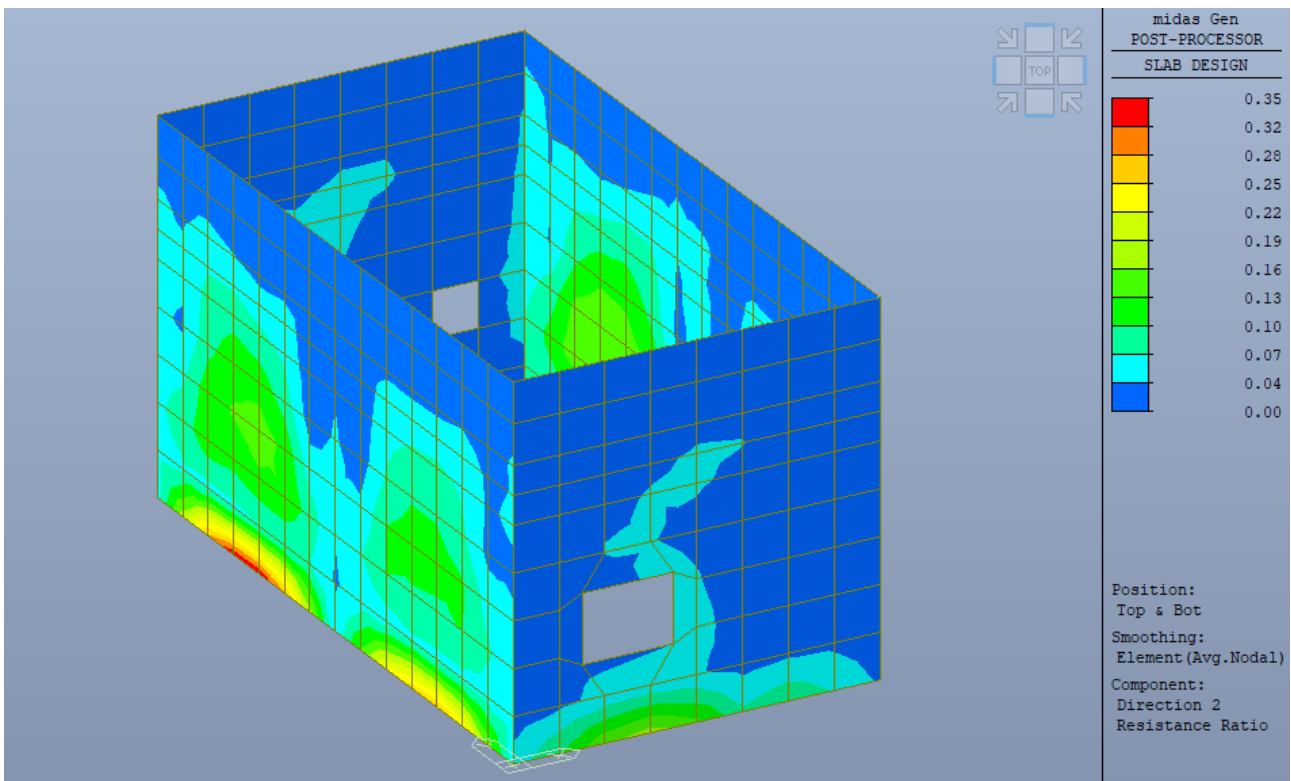
Plata - Indici di resistenza a flessione direzione X (inviluppo SLU e SLV)



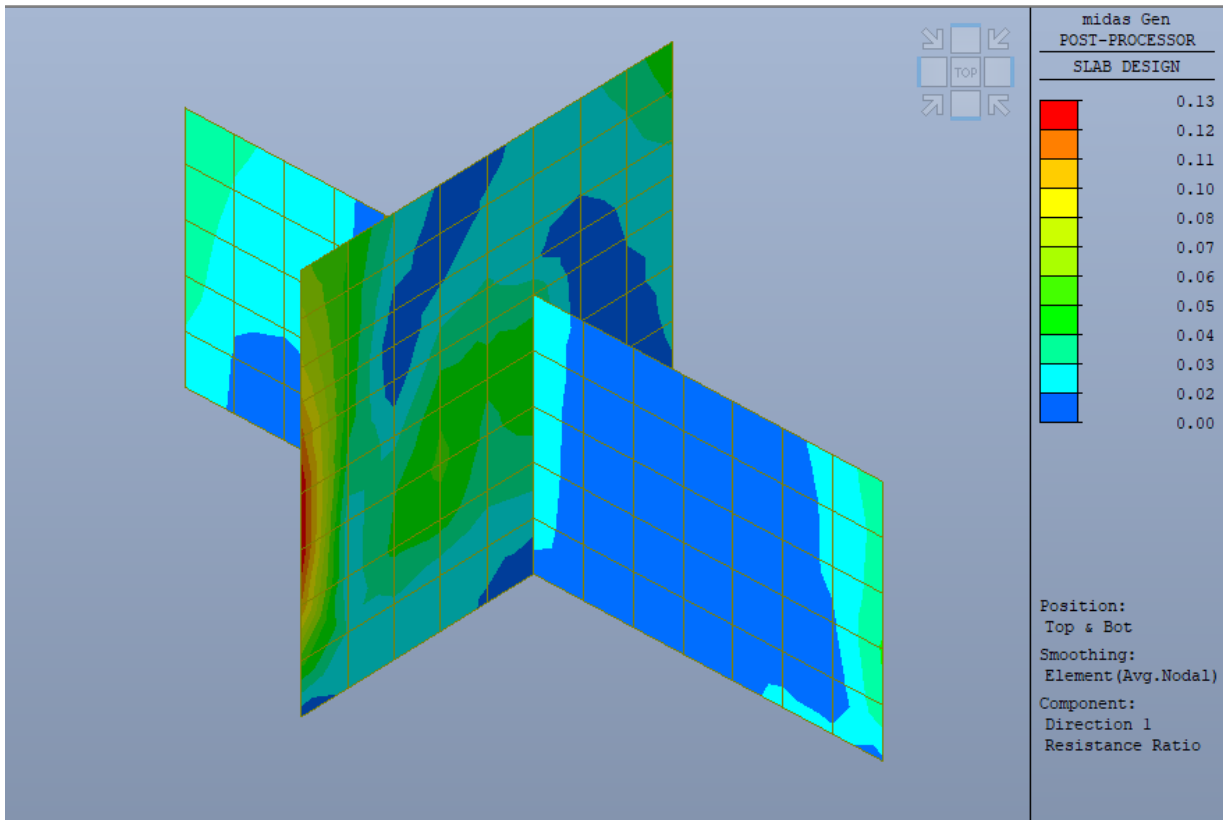
Plata - Indici di resistenza a flessione direzione Y (inviluppo SLU e SLV)



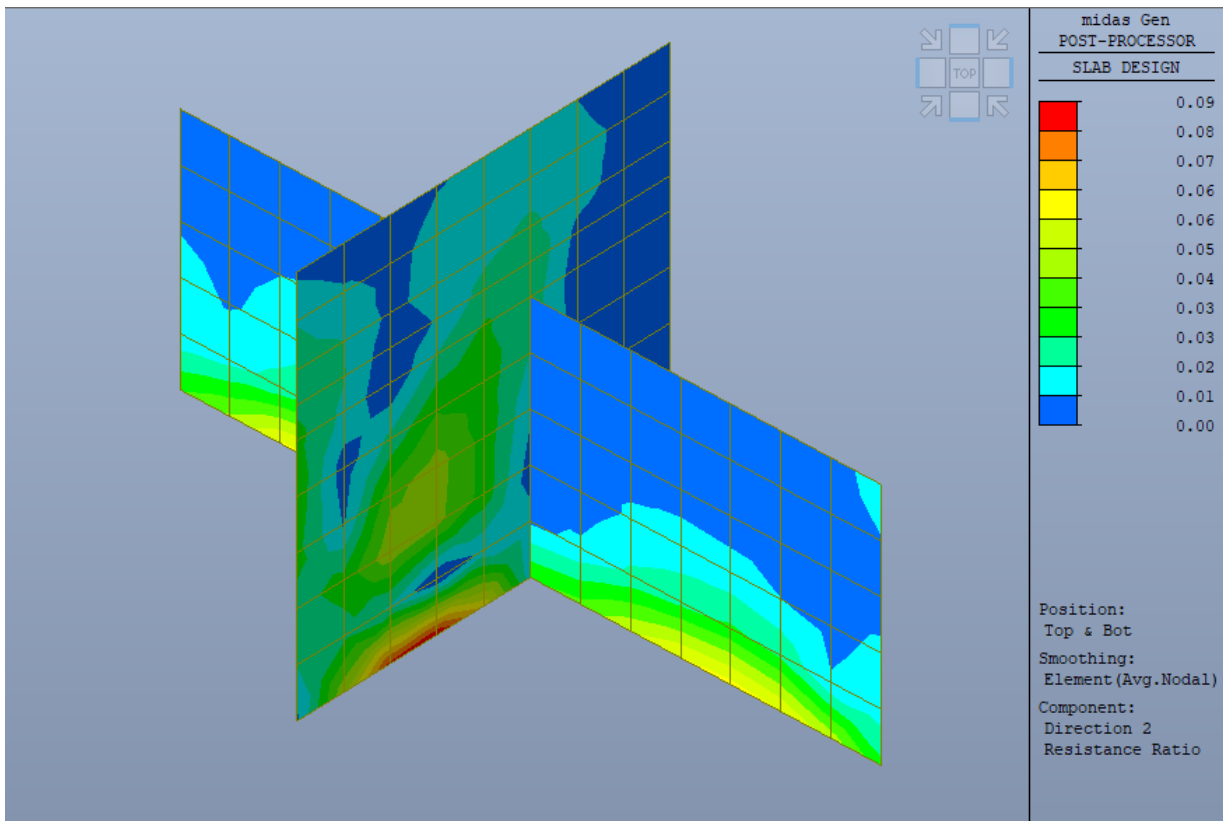
Pareti esterne - Indici di resistenza a flessione direzione orizzontale (involuppo SLU e SLV)



Pareti esterne - Indici di resistenza a flessione direzione verticale (involuppo SLU e SLV)



Pareti interne - Indici di resistenza a flessione direzione orizzontale (involuppo SLU e SLV)



Pareti interne - Indici di resistenza a flessione direzione verticale (involuppo SLU e SLV)

1.8 Verifiche di resistenza SLU analitiche

1.8.1 Verifiche Platea

```
=====  
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Platea, Dir 1.  
=====
```

```
-----  
Thk Elem POS AsReq AsUse | M_Ed( LCB) M_Rd Rat CHK
```

```
-----  
0.3000 357 BOT 0.0004 0.0006 | 17.8551( 8) 54.0653 0.330 OK
```

```
371 TOP 0.0004 0.0006 | 9.76843( 3) 54.0653 0.181 OK  
-----
```

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 357

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0500 m.

dT = 0.0500 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2500 m.

lambda = 0.800

a = lambda * x = 0.010 m.

$$\eta = 1.000$$

$$C_c = \eta \cdot f_{cd} \cdot b \cdot a = 0.2208 \text{ kN.}$$

$$M_{Rd} = C_c \cdot (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (\quad 0.0004 \text{ m}^2/\text{m.})$$

$$M_{Ed} = 17.8551 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.330 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \quad (f_{ck} \leq 50 \text{ MPa.})$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 371

Thickness : 0.3000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0500 \text{ m.}$

$$d_T = 0.0500 \text{ m.}$$

LCB No. : 3

-. Information of Design.

$$b = 0.0010 \text{ m.} \quad (\text{by Code Unit Length}).$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2208 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (\quad 0.0004 \text{ m}^2/\text{m.})$$

$$M_{Ed} = 9.7684 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.181 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \quad (f_{ck} \leq 50 \text{ MPa.})$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Platea, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 380 BOT 0.0004 0.0006 | 12.4916(8) 51.6656 0.242 OK

371 TOP 0.0004 0.0006 | 7.05558(3) 51.6656 0.137 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 380

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0600 m.

dT = 0.0600 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2400 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2200 kN.

M_Rd = Cc*(d-a/2) = 51.6656 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 12.4916 kN-m./m.

M_Rd = 51.6656 kN-m./m.

RatM = M_Ed / M_Rd = 0.242 < 1.0 ----> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

Limit(x/d) = 0.450 (fck <= 50 MPa.)

x/d ratio = 0.040/ 0.450 = 0.089 ----> O.K

<< TOP >>

-. Information of Parameters.

Elem No. : 371

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0600 m.

dT = 0.0600 m.

LCB No. : 3

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2400 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2200 kN.

M_Rd = Cc*(d-a/2) = 51.6656 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 7.0556 kN-m./m.

M_Rd = 51.6656 kN-m./m.

RatM = M_Ed / M_Rd = 0.137 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.



$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

1.8.2 Verifiche Pareti Esterne

=====

[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 1, Dir 1.

=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 56 BOT 0.0004 0.0006 | 4.39751(4) 54.0653 0.081 OK

41 TOP 0.0004 0.0006 | 10.2473(4) 54.0653 0.190 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 56

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0500 m.

dT = 0.0500 m.

LCB No. : 4

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2500 m.

$$\lambda = 0.800$$

$$a = \lambda \cdot x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta \cdot f_{cd} \cdot b \cdot a = 0.2208 \text{ kN.}$$

$$M_{Rd} = C_c \cdot (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (\quad 0.0004 \text{ m}^2/\text{m.})$$

$$M_{Ed} = 4.3975 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.081 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \quad (f_{ck} \leq 50 \text{ MPa.})$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 41

Thickness : 0.3000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0500 \text{ m.}$

$$d_T = 0.0500 \text{ m.}$$

LCB No. : 4



-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2208 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 10.2473 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.190 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 2, Dir 1.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 166 BOT 0.0004 0.0006 | 9.53534(8) 54.0653 0.176 OK

185 TOP 0.0004 0.0006 | 14.3719(8) 54.0653 0.266 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 166

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0500 m.

dT = 0.0500 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2500 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2208 kN.

M_Rd = Cc*(d-a/2) = 54.0653 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 9.5353 kN-m./m.

M_Rd = 54.0653 kN-m./m.

RatM = M_Ed / M_Rd = 0.176 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 185

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0500 m.

$$dT = 0.0500 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$Cc = \eta * fcd * b * a = 0.2208 \text{ kN.}$$

$$M_{Rd} = Cc * (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 14.3719 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$\text{RatM} = M_{Ed} / M_{Rd} = 0.266 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ----> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 3, Dir 1.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 296 BOT 0.0004 0.0006 | 12.5511(8) 54.0653 0.232 OK

285 TOP 0.0004 0.0006 | 5.95332(8) 54.0653 0.110 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 296

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0500 m.

$$dT = 0.0500 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda \cdot x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta \cdot f_{cd} \cdot b \cdot a = 0.2208 \text{ kN.}$$

$$M_{Rd} = C_c \cdot (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (\quad 0.0004 \text{ m}^2/\text{m.})$$

$$M_{Ed} = 12.5511 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.232 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \quad (f_{ck} \leq 50 \text{ MPa.})$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 285

Thickness : 0.3000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0500 \text{ m.}$

$$d_T = 0.0500 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2208 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 5.9533 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.110 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 4, Dir 1.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 80 BOT 0.0004 0.0006 | 13.2508(8) 54.0653 0.245 OK

96 TOP 0.0004 0.0006 | 8.25272(8) 54.0653 0.153 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 80

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0500 m.

dT = 0.0500 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2500 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2208 kN.

M_Rd = Cc*(d-a/2) = 54.0653 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 13.2508 kN-m./m.

M_Rd = 54.0653 kN-m./m.

RatM = M_Ed / M_Rd = 0.245 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 96

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0500 m.

$$dT = 0.0500 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$Cc = \eta * fcd * b * a = 0.2208 \text{ kN.}$$

$$M_{Rd} = Cc * (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 8.2527 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$\text{RatM} = M_{Ed} / M_{Rd} = 0.153 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 1, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 59 BOT 0.0004 0.0006 | 2.59005(4) 51.6656 0.050 OK

52 TOP 0.0004 0.0006 | 9.04193(4) 51.6656 0.175 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 59

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0600 m.

$$dT = 0.0600 \text{ m.}$$

LCB No. : 4

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (\quad 0.0004 \text{ m}^2/\text{m.})$$

$$M_{Ed} = 2.5900 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$\text{RatM} = M_{Ed} / M_{Rd} = 0.050 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \quad (f_{ck} \leq 50 \text{ MPa.})$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 52

Thickness : 0.3000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0600 \text{ m.}$

$$d_T = 0.0600 \text{ m.}$$

LCB No. : 4



-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 9.0419 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.175 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 2, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 174 BOT 0.0004 0.0006 | 8.07321(8) 51.6656 0.156 OK

202 TOP 0.0004 0.0006 | 14.7274(8) 51.6656 0.285 OK

<< BOTTOM >

-. Information of Parameters.

Elem No. : 174

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0600 m.

dT = 0.0600 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2400 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2200 kN.

M_Rd = Cc*(d-a/2) = 51.6656 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 8.0732 kN-m./m.

M_Rd = 51.6656 kN-m./m.

RatM = M_Ed / M_Rd = 0.156 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 202

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0600 m.

$$dT = 0.0600 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$Cc = \eta * fcd * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = Cc * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 14.7274 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$\text{RatM} = M_{Ed} / M_{Rd} = 0.285 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 3, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 282 BOT 0.0004 0.0006 | 12.3696(8) 51.6656 0.239 OK

283 TOP 0.0004 0.0006 | 4.77083(8) 51.6656 0.092 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 282

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0600 m.

$$dT = 0.0600 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m.} \quad (\quad 0.0004 \text{ m}^2/\text{m.})$$

$$M_{Ed} = 12.3696 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.239 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \quad (f_{ck} \leq 50 \text{ MPa.})$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 283

Thickness : 0.3000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0600 \text{ m.}$

$$d_T = 0.0600 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 4.7708 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.092 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete EXT 4, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 97 BOT 0.0004 0.0006 | 17.9335(8) 51.6656 0.347 OK

100 TOP 0.0004 0.0006 | 7.41982(8) 51.6656 0.144 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 97

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0600 m.

dT = 0.0600 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2400 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2200 kN.

M_Rd = Cc*(d-a/2) = 51.6656 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 17.9335 kN-m./m.

M_Rd = 51.6656 kN-m./m.

RatM = M_Ed / M_Rd = 0.347 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 100

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0600 m.

$$dT = 0.0600 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * fcd * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 7.4198 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$\text{RatM} = M_{Ed} / M_{Rd} = 0.144 < 1.0 \text{ ---> O.K !}$$



- Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

1.8.3 Verifiche Pareti Interne

=====

[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete INT 1, Dir 1.

=====

Thk	Elem	POS	AsReq	AsUse	M_Ed(LCB)	M_Rd	Rat	CHK
-----	------	-----	-------	-------	------------	------	-----	-----

0.2000	441	BOT	0.0003	0.0006	1.27291(3)	31.8656	0.040	OK
--------	-----	-----	--------	--------	-------------	---------	-------	----

448	TOP	0.0003	0.0006	1.21734(8)	31.8656	0.038	OK
-----	-----	--------	--------	-------------	---------	-------	----

<< BOTTOM >>

- Information of Parameters.

Elem No. : 441

Thickness : 0.2000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0500 m.

$$dT = 0.0500 \text{ m.}$$

LCB No. : 3

- Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.1500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 31.8656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0003 \text{ m}^2/\text{m. (} 0.0003 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 1.2729 \text{ kN-m./m.}$$

$$M_{Rd} = 31.8656 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.040 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 448

Thickness : 0.2000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0500 \text{ m.}$

$$d_T = 0.0500 \text{ m.}$$



LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.1500 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2200 kN.

M_Rd = Cc*(d-a/2) = 31.8656 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0003 m²/m. (0.0003 m²/m.)

M_Ed = 1.2173 kN-m./m.

M_Rd = 31.8656 kN-m./m.

RatM = M_Ed / M_Rd = 0.038 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

Limit(x/d) = 0.450 (fck <= 50 MPa.)

x/d ratio = 0.040/ 0.450 = 0.089 ---> O.K

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete INT 2, Dir 1.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 235 BOT 0.0004 0.0006 | 2.88008(8) 54.0653 0.053 OK

220 TOP 0.0004 0.0006 | 7.07968(3) 54.0653 0.131 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 235

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0500 m.

dT = 0.0500 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2500 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2208 kN.

M_Rd = Cc*(d-a/2) = 54.0653 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 2.8801 kN-m./m.

M_Rd = 54.0653 kN-m./m.

RatM = M_Ed / M_Rd = 0.053 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ----> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 220

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0500 m.

$$dT = 0.0500 \text{ m.}$$

LCB No. : 3

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2500 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$Cc = \eta * fcd * b * a = 0.2208 \text{ kN.}$$

$$M_{Rd} = Cc * (d - a/2) = 54.0653 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 7.0797 \text{ kN-m./m.}$$

$$M_{Rd} = 54.0653 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.131 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete INT 1, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.2000 443 BOT 0.0002 0.0006 | 0.45256(3) 29.5792 0.015 OK

459 TOP 0.0002 0.0006 | 1.78536(20) 29.5792 0.060 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 443

Thickness : 0.2000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0600 m.

dT = 0.0600 m.

LCB No. : 3

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.1400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2193 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 29.5792 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0002 \text{ m}^2/\text{m. (} 0.0002 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 0.4526 \text{ kN-m./m.}$$

$$M_{Rd} = 29.5792 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.015 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 459

Thickness : 0.2000 m.

Materials : $f_{ck} = 32000.0000 \text{ KPa.}$

$$f_{cd} = 21333.3333 \text{ KPa.}$$

$$f_{yk} = 450000.0000 \text{ KPa.}$$

Covering : $d_B = 0.0600 \text{ m.}$

$$d_T = 0.0600 \text{ m.}$$



LCB No. : 20



-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.1400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$C_c = \eta * f_{cd} * b * a = 0.2193 \text{ kN.}$$

$$M_{Rd} = C_c * (d - a/2) = 29.5792 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0002 \text{ m}^2/\text{m. (} 0.0002 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 1.7854 \text{ kN-m./m.}$$

$$M_{Rd} = 29.5792 \text{ kN-m./m.}$$

$$RatM = M_{Ed} / M_{Rd} = 0.060 < 1.0 \text{ ---> O.K !}$$

-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

$$\text{Limit}(x/d) = 0.450 \text{ (} f_{ck} \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

=====
[[[*]]] SLAB DESIGN MAXIMUM RESULT DATA : DOMAIN 1-Parete INT 2, Dir 2.
=====

Thk Elem POS AsReq AsUse | M_Ed(LCB) M_Rd Rat CHK

0.3000 229 BOT 0.0004 0.0006 | 2.41111(8) 51.6656 0.047 OK

232 TOP 0.0004 0.0006 | 4.49748(8) 51.6656 0.087 OK

<< BOTTOM >>

-. Information of Parameters.

Elem No. : 229

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

fcd = 21333.3333 KPa.

fyk = 450000.0000 KPa.

Covering : dB = 0.0600 m.

dT = 0.0600 m.

LCB No. : 8

-. Information of Design.

b = 0.0010 m. (by Code Unit Length).

d = 0.2400 m.

lambda = 0.800

a = lambda * x = 0.010 m.

eta = 1.000

Cc = eta*fcd*b*a = 0.2200 kN.

M_Rd = Cc*(d-a/2) = 51.6656 kN-m./m.

-. Information of Moments and Result.

Rein. Bar : P12 @200

As_req = 0.0004 m²/m. (0.0004 m²/m.)

M_Ed = 2.4111 kN-m./m.

M_Rd = 51.6656 kN-m./m.

RatM = M_Ed / M_Rd = 0.047 < 1.0 ---> O.K !

-. Check ratio of neutral axis depth to effective depth.

x/d = 0.040

$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

<< TOP >>

-. Information of Parameters.

Elem No. : 232

Thickness : 0.3000 m.

Materials : fck = 32000.0000 KPa.

$$fcd = 21333.3333 \text{ KPa.}$$

$$fyk = 450000.0000 \text{ KPa.}$$

Covering : dB = 0.0600 m.

$$dT = 0.0600 \text{ m.}$$

LCB No. : 8

-. Information of Design.

$$b = 0.0010 \text{ m. (by Code Unit Length).}$$

$$d = 0.2400 \text{ m.}$$

$$\lambda = 0.800$$

$$a = \lambda * x = 0.010 \text{ m.}$$

$$\eta = 1.000$$

$$Cc = \eta * fcd * b * a = 0.2200 \text{ kN.}$$

$$M_{Rd} = Cc * (d - a/2) = 51.6656 \text{ kN-m./m.}$$

-. Information of Moments and Result.

Rein. Bar : P12 @200

$$A_{s_req} = 0.0004 \text{ m}^2/\text{m. (} 0.0004 \text{ m}^2/\text{m.)}$$

$$M_{Ed} = 4.4975 \text{ kN-m./m.}$$

$$M_{Rd} = 51.6656 \text{ kN-m./m.}$$

$$\text{RatM} = M_{Ed} / M_{Rd} = 0.087 < 1.0 \text{ ---> O.K !}$$

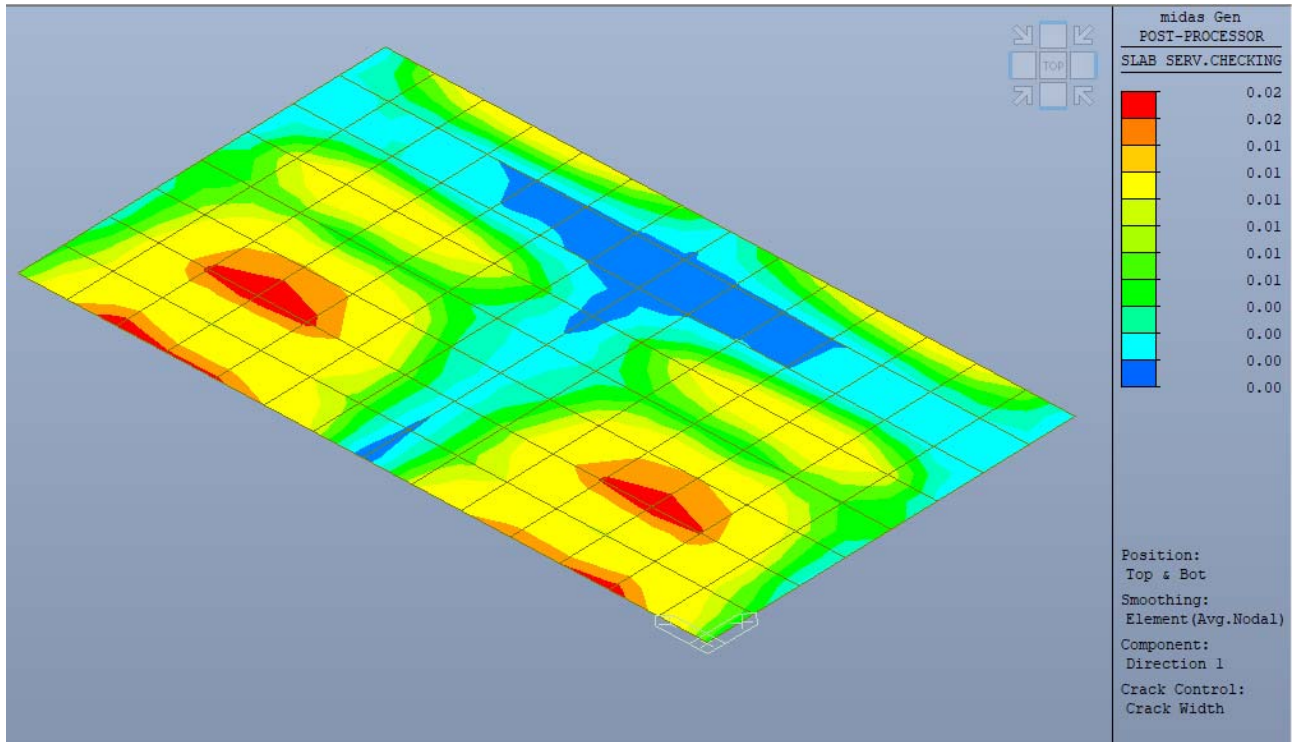
-. Check ratio of neutral axis depth to effective depth.

$$x/d = 0.040$$

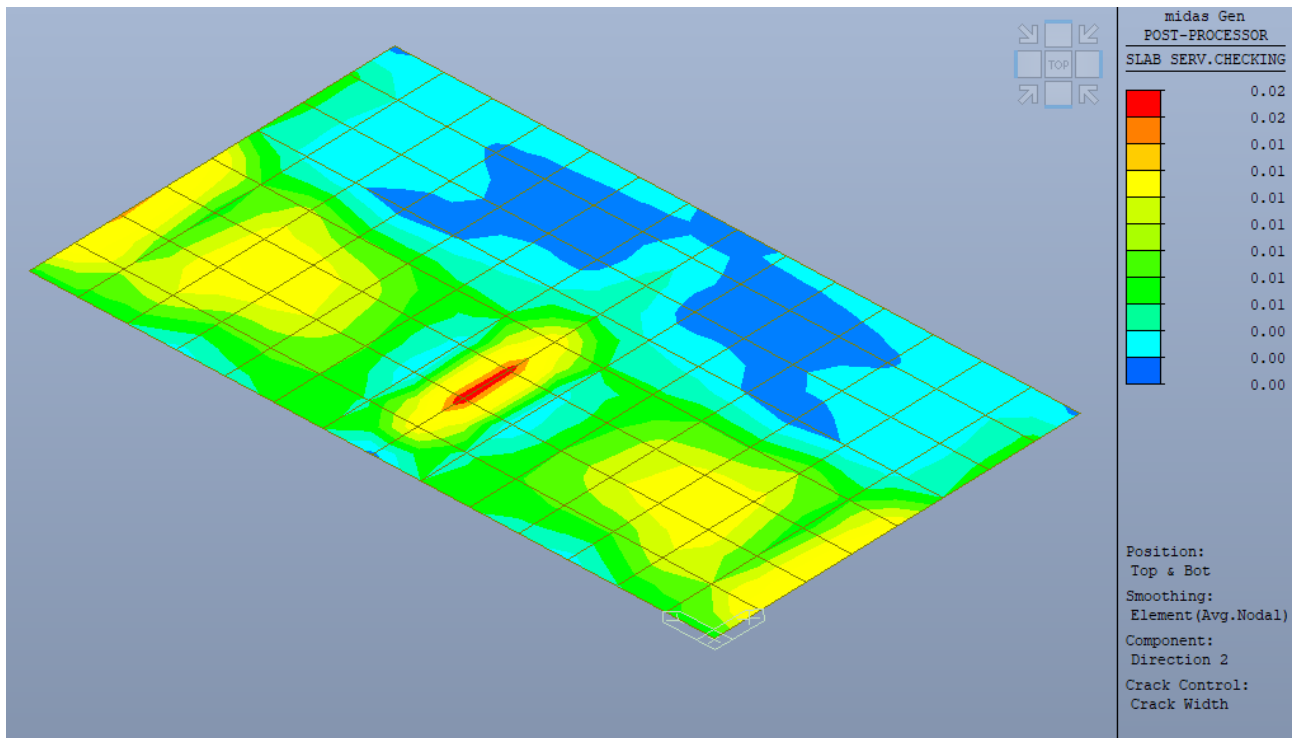
$$\text{Limit}(x/d) = 0.450 \text{ (fck } \leq 50 \text{ MPa.)}$$

$$x/d \text{ ratio} = 0.040 / 0.450 = 0.089 \text{ ---> O.K}$$

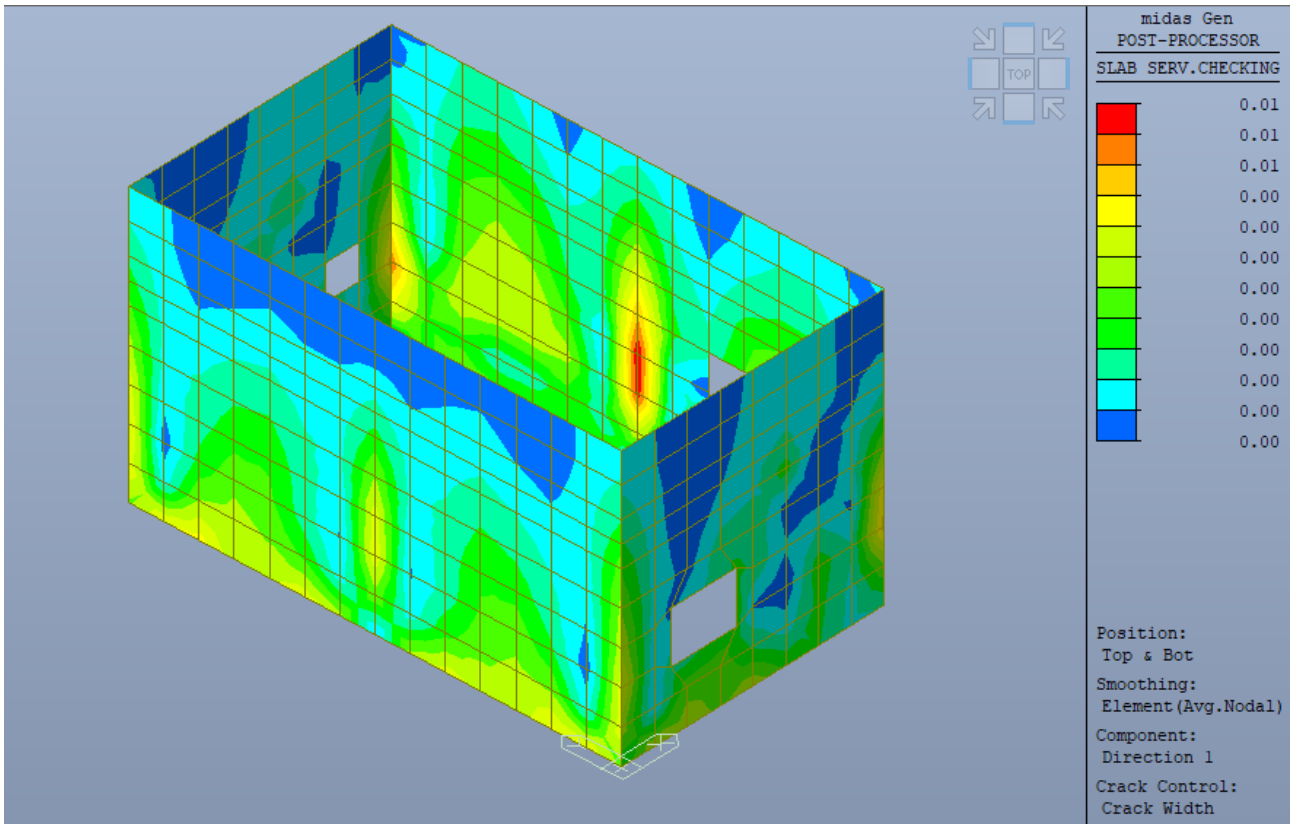
1.9 Verifiche in condizioni di esercizio SLE



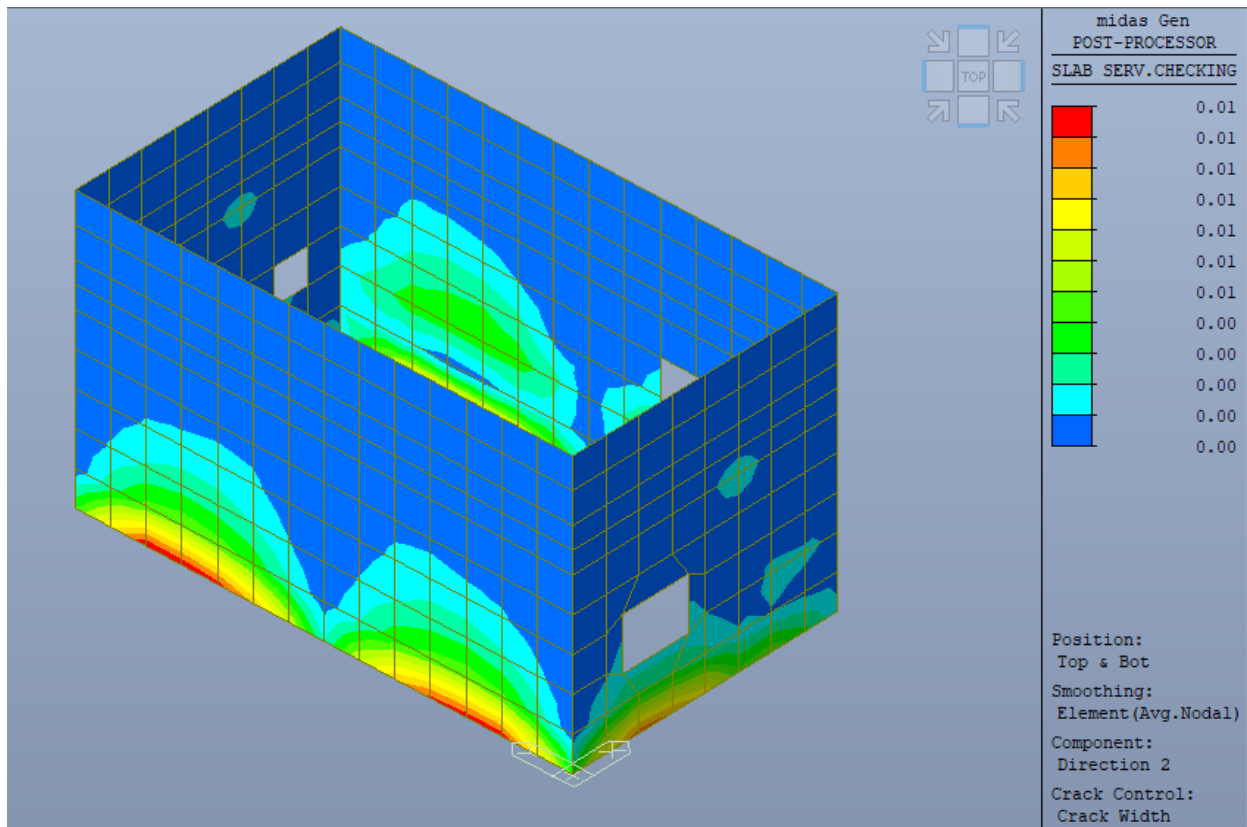
Platea – Verifica a fessurazione SLE - ratio direzione Y



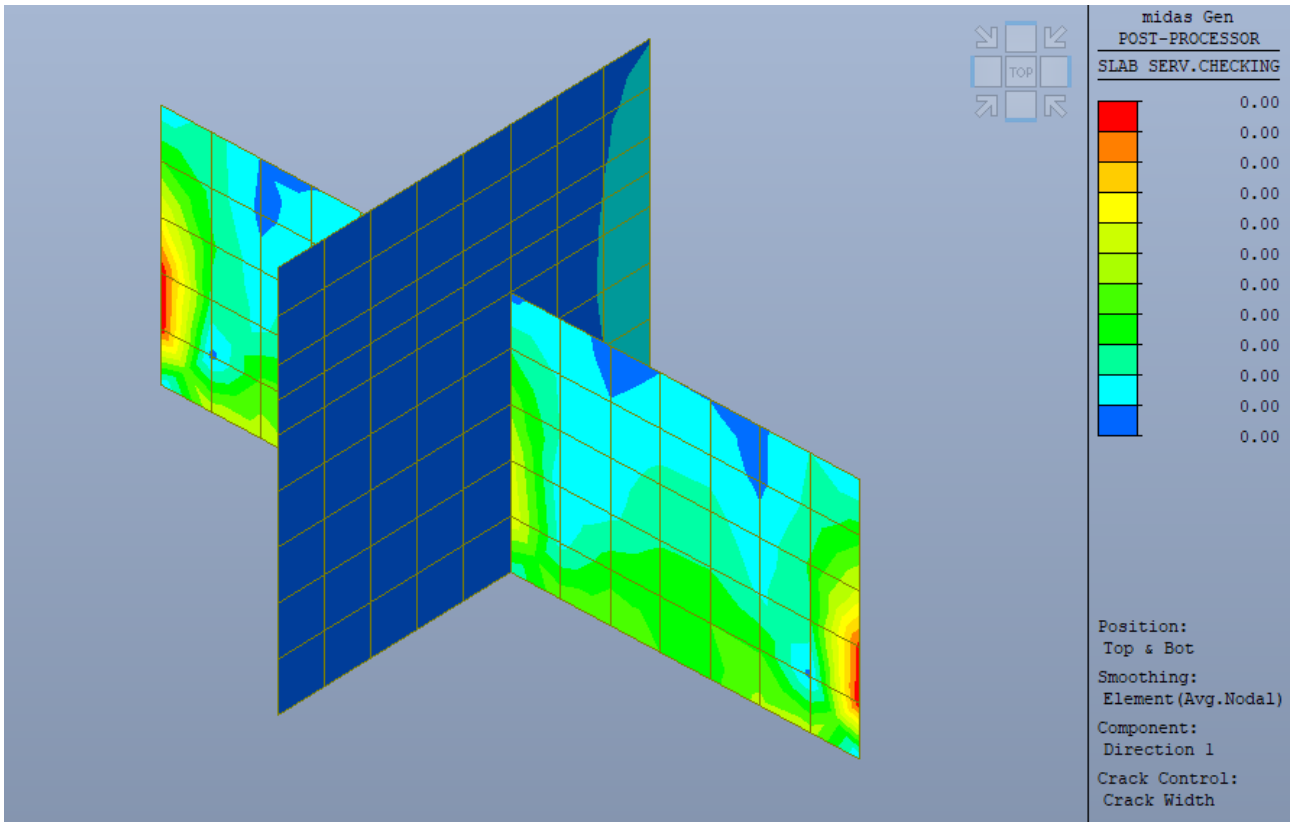
Platea – Verifica a fessurazione SLE - ratio direzione X



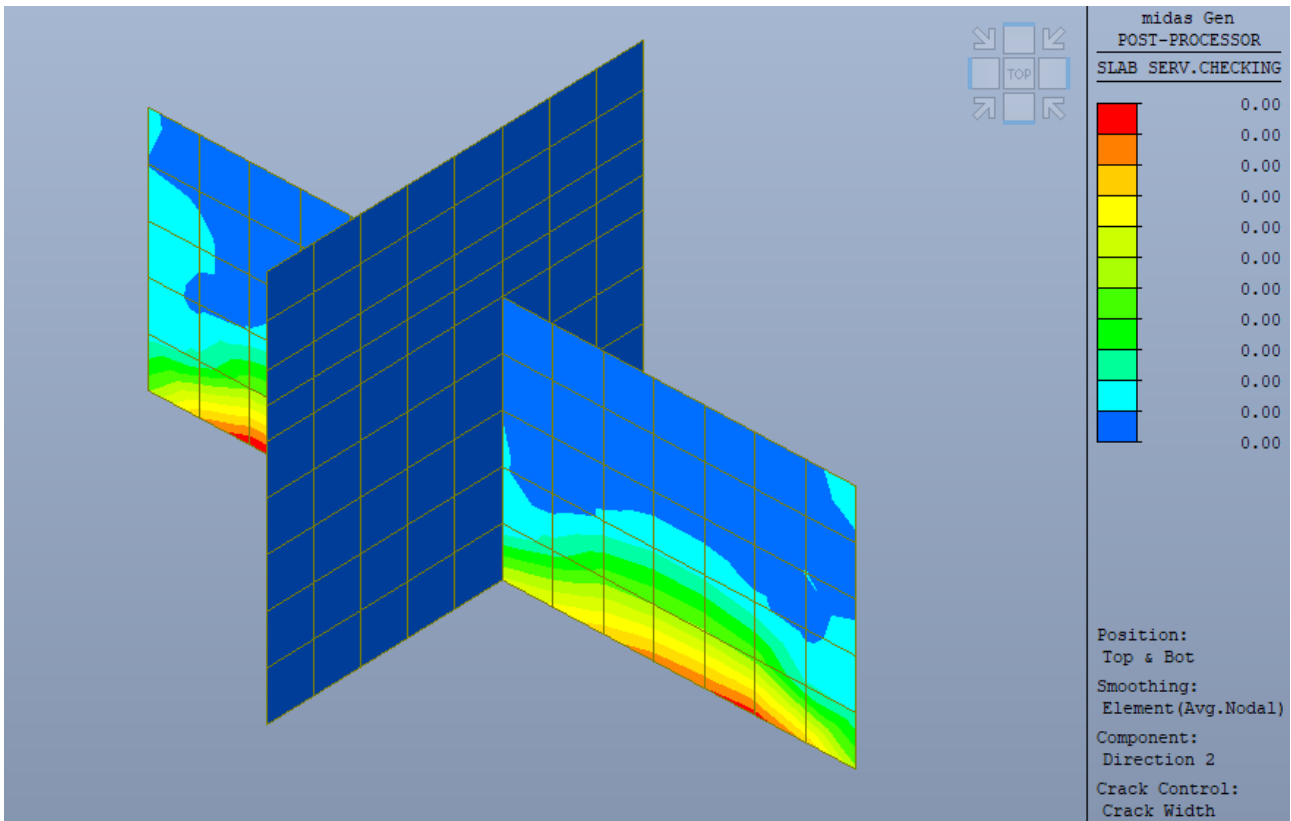
Pareti esterne – Verifica a fessurazione SLE - ratio direzione X



Pareti esterne – Verifica a fessurazione SLE - ratio direzione Y



Pareti interne – Verifica a fessurazione SLE - ratio direzione X



Pareti interne – Verifica a fessurazione SLE - ratio direzione Y